

THE TERRACES OF LAFAYETTE
TRAFFIC IMPACT STUDY

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I. INTRODUCTION AND SUMMARY OF FINDINGS

A. Introduction

This Traffic Impact Study (“Study”) reviews the potential traffic impacts of The Terraces of Lafayette, a proposed multifamily residential apartment project, consisting of 315 units, with a mixture of one, two and three bedroom apartments (the “Project”). As shown on **Figure 1/Project Location**, the Project site is located at the western terminus of Deer Hill Road, at the northwest corner of the Highway 24/Pleasant Hill Road intersection.

As shown on **Figure 2/Site Plan**, the Project site consists of approximately 22 acres. The proposed apartments are contained within two and three-story building structures, identified as “A” through “M,” predominantly on the “flat shelf” area of the Project site. The Project includes 569 on-site parking spaces. The main Project entrance is off Pleasant Hill Road, with two secondary entrances off Deer Hill road, and with interior circulation as shown on **Figure 2/Site Plan**.

The Project applicant has proposed significant road and circulation improvements to address traffic circulation on Pleasant Hill Road. These road and circulation improvements include: (i) construction of a northbound turn lane on Pleasant Hill Road, enabling vehicles to turn left into the main Project entrance; and (ii) a new southbound through-lane on Pleasant Hill Road from north of Deer Hill to the Highway 24 freeway on ramp. As detailed in this Study, the proposed northbound turn lane virtually eliminates the addition of Project vehicles from turning left at Deer Hill Road during the PM peak hour. Further, the proposed southbound through-lane on Pleasant Hill Road will result in a significant increase in capacity during the AM peak hour.

This Study: (i) describes the existing traffic and circulation system, parking conditions, and pedestrian and transit conditions in the vicinity of the Project, together with existing City and regional policies and standards regarding traffic impacts; and (ii) provides an analysis of the potential traffic impacts of the Project. This Study has been conducted consistent with the requirements and methodologies required by the City of Lafayette.

B. Summary of Findings

While a total of eight (8) intersections and several roadways are analyzed in this Study, it is clear that the area with the greatest potential for impacts from Project traffic is along Pleasant Hill Road, and specifically the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection. Currently, the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection operates at an acceptable Level of Service during both the AM and PM peak hours. However, there is a concern regarding future traffic travelling southbound through the intersection during the AM peak hour, and northbound through the intersection during the PM peak hour. Without the Project, such increasing traffic (from regional traffic growth and future build-out within the City) is expected to eventually degrade the referenced Level of Service.

The construction of the Project (including proposed traffic-related improvements) is not expected to have any significant adverse impact on traffic levels and/or the current or future Levels of Service at the studied intersections. Additionally, the Project-related traffic

improvements will actually expand the capacity of Pleasant Hill Road to better accommodate cumulative future traffic increases.

The Project, with the proposed road and circulation improvements, will significantly improve the traffic conditions along Pleasant Hill Road and at the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection. This is primarily due to the construction of the additional southbound through-lane along Pleasant Hill Road.

The Project, with the proposed road and circulation improvements, is consistent with the standards set by the Contra Costa Transportation Authority, and specifically the Multi-modal Transportation Service Objectives (MTSOs) as listed in the Lamorinda Action Plan and the Pleasant Hill Road Action Plan.

For these and other reasons set forth in this Study, the Project traffic will not have a significant adverse effect or impact on existing and future traffic conditions.

II. CURRENT PROJECT SETTING

This section of the Study describes the roadways, traffic conditions and other existing transportation characteristics in the vicinity of the Project. The primary basis of the analysis is the peak hour level of service for the key intersections identified herein. The hours identified as the “peak” hours are generally between 8:00 a.m. and 9:00 a.m. and 5:00 p.m. and 6:00 p.m. for all of the transportation facilities described. Throughout this Study, these peak hours will be identified as the AM and PM peak hours, respectively.

A. Project Study Intersections

A traffic study must be prepared for the Contra Costa Transportation Authority (CCTA) for all projects that generate over 100 trips during a one-hour period. With 315 residential units it is estimated that the proposed Project could generate as many as 185 vehicle trips on a weekday during the critical PM peak hour. Based on the project’s trip generation and the potential for traffic impacts, the study area was defined, and a preliminary list of Project study intersections was prepared.

Generally, the efficiency of street systems can be objectively measured by focusing on conditions at identified “key intersections” – those intersections which are potentially impacted by Project-generated traffic. Eight such key intersections are set forth below. (Project traffic generation at intersections other than the noted key intersections is projected at fewer than 50 trips during the peak hour, thereby requiring no further capacity analysis.)

The key intersections that are analyzed in this Study are identified in **Figure 1/Project Location** as follows:

- Intersection #1: Pleasant Hill Road and Rancho View Drive
- Intersection #2: Pleasant Hill Road and Green Valley Drive
- Intersection #3: Pleasant Hill Road and Reliez Valley Road
- Intersection #4: Pleasant Hill Road and Springhill Road/Quandt Road
- Intersection #5: Pleasant Hill Road and Deer Hill Road/Stanley Boulevard
- Intersection #6: Pleasant Hill Road and Mt. Diablo Blvd/Hwy 24 EB On-Ramp
- Intersection #7: Pleasant Hill Road and Old Tunnel Road/Hwy 24 EB Off-Ramp
- Intersection #8: Deer Hill Road and Brown Avenue

All of these intersections are signalized with the exception of Brown Avenue.

B. Traffic Analysis Scenarios

The key intersections were evaluated for the following scenarios:

- Scenario 1: Existing Conditions – Level of Service (LOS) based on existing peak hour volumes and existing intersection configurations.
- Scenario 2: Baseline Conditions – Existing traffic plus anticipated traffic from approved developments in the study area.¹
- Scenario 3: Baseline Conditions Plus Project – Baseline conditions peak hour volumes plus trips from the Project with the Project’s road and circulation improvements.
- Scenario 4: Cumulative Conditions (Year 2030) per the Certified General Plan EIR (without the Project; and with the Project).

C. Existing Roadway Network

The project location and the surrounding roadway network are illustrated in **Figure 1/Project Location**. The primary roadways that would be affected by the project include:

- **Pleasant Hill Road** – Pleasant Hill Road is a four-lane arterial roadway located on the east side of Lafayette. It has an interchange with the State Highway 24 freeway just south of the Project.
- **Deer Hill Road** – Deer Hill Road is a two-lane roadway that travels generally in a west-east direction, located on the north side of Highway 24. At Pleasant Hill Road, the name changes to Stanley Boulevard which provides the principal access to Acalanes High School.
- **Mt. Diablo Boulevard** – Mt Diablo Boulevard is a four-lane arterial on the south side of Highway 24, and connects with Lafayette’s downtown commercial district.

¹ This Study assumes approximately four (4) years of growth for Scenario 2, as opposed to growth to the year 2030 in Scenario 4.

- Other streets that appear in this traffic analysis include **Springhill Road, Quandt Road, Reliez Valley Road, Green Valley Drive, Rancho View Drive, Brown Avenue** and **Old Tunnel Road**. These are all two-lane local streets serving primarily residential uses. Brown Avenue also includes some commercial development and an underpass crossing of Highway 24, where it connects to Mt. Diablo Boulevard in Downtown Lafayette.

III. INTERSECTION ANALYSIS METHODOLOGY

A. Highway Capacity Manual (HCM) Level of Service Methodology

Existing operational conditions at the eight key intersections have been evaluated using Synchro Software to implement the 2000 *Highway Capacity Manual (HCM)* Level of Service (LOS) methodology.² “Level of Service” is an expression, in the form of a scale, of the relationship between the capacity of an intersection (or roadway segment) to accommodate the volume of traffic moving through the intersection at any given time. The Level of Service scale describes traffic flow with six ratings ranging from “A” through “F,” with “A” indicating relatively free flow of traffic and “F” indicating stop-and-go traffic characterized by traffic jams.

As the amount of traffic moving through a given intersection or roadway segment increases, the traffic flow conditions that motorists experience rapidly deteriorate as the capacity of the intersection or roadway segment is reached. Under such conditions, there is general instability in the traffic flow, which means that relatively small incidents (e.g., momentary engine stall) can cause considerable fluctuations in speeds and delays that lead to traffic congestion. This near-capacity situation is labeled Level of Service (LOS) “E.” Beyond LOS “E,” the intersection or roadway segment capacity has been exceeded, and arriving traffic will exceed the ability of the intersection to accommodate the traffic.

For *signalized intersections*, the HCM methodology determines the capacity of each lane group approaching the intersection. The LOS is then based on average control delay (in seconds per vehicle) for the various movements within the intersection. A combined weighted average control delay and LOS are presented for the intersection.

Table 1 summarizes the relationship between LOS and average control delay at signalized intersections, using the HCM methodology as set forth in **Appendix Section 1**. Please note that, per the requirements set forth by the Contra Costa County Transportation Authority (CCTA), all signalized intersections have also been analyzed using the methodology set forth in the Final Technical Procedures Update (dated July 19, 2006). The CCTA LOS printouts have been included in **Appendix Section 2** to allow verification that these results are the same or better when compared to the HCM results.

² *Highway Capacity Manual*, Transportation Research Board, Washington D.C., 2000. As part of the *HCM methodology*, adjustments are typically made for various factors that reduce the ability of the streets to accommodate vehicles (such as the downtown nature of the area, number of pedestrians, vehicle types, lane widths, grades, on-street parking and queues). These adjustments are performed to ensure that the LOS analysis results reflect the operating conditions that are observed in the field.

For *unsignalized intersections* (all-way stop controlled and two-way stop controlled), the average control delay and LOS operating conditions are calculated by: (i) approach (e.g., northbound) and (ii) movement (e.g., northbound left-turn) for those movements that are subject to delay. In general, the operating conditions for unsignalized intersections are presented for the worst approach. **Table 2** summarizes the relationship between LOS and average control delay at unsignalized intersections, using the HCM methodology.

TABLE 1
SIGNALIZED INTERSECTION LEVEL OF SERVICE DEFINITIONS (HCM)

| <u>Level of Service</u> | <u>Description of Operations</u> | <u>Average Delay (sec/veh)</u> |
|--------------------------------|--|---------------------------------------|
| A | Insignificant Delays: No approach phase is fully used and no vehicle waits longer than one red indication. | ≤ 10 |
| B | Minimal Delays: An occasional approach phase is fully used. Drivers begin to feel restricted. | > 10 to 20 |
| C | Acceptable Delays: Major approach phase may become fully used. Most drivers feel somewhat restricted. | > 20 to 35 |
| D | Tolerable Delays: Drivers may wait through no more than one red indication. Queues may develop but dissipate rapidly without excessive delays. | > 35 to 55 |
| E | Significant Delays: Volumes approaching capacity. Vehicles may wait through several signal cycles and long vehicle queues from upstream. | > 55 to 80 |
| F | Excessive Delays: Represents conditions at capacity, with extremely long delays. Queues may block upstream intersections. | > 80 |

SOURCE: *Highway Capacity Manual*, Transportation Research Board, 2000

TABLE 2
UNSIGNALIZED INTERSECTION LEVEL OF SERVICE DEFINITIONS (HCM)

| <u>Level of Service</u> | <u>Description of Operations</u> | <u>Average Delay (secs/veh)</u> |
|--------------------------------|--|--|
| A | No delay for stop-controlled approaches. | 0 to 10 |
| B | Operations with minor delays. | > 10 to 15 |
| C | Operations with moderate delays. | > 15 to 25 |
| D | Operations with some delays. | > 25 to 35 |
| E | Operations with high delays and long queues. | > 35 to 50 |
| F | Operation with extreme congestion, with very high delays and long queues unacceptable to most drivers. | > 50 |

SOURCE: *Highway Capacity Manual*, Transportation Research Board, 2000

B. The City of Lafayette Level of Service Policy

The City of Lafayette General Plan, adopted in 2002 (the “General Plan”) confirms that the City uses both the Contra Costa Transportation Authority (CCTA) and the Highway Capacity Manual (HCM) methods for calculation LOS on roadways and intersections.³ Because it has been found that in Lafayette the HCM procedures produce a more accurate estimate of actual operating conditions, it is the preferred methodology.

The length of time a vehicle is delayed at a signalized intersection as used in this Study is based on the year 2000 HCM. The HCM procedures used in the 2002 Lafayette General Plan were based on the 1994 HCM. The resultant LOS letter grade is the same using either HCM.

C. Contra Costa Transportation Authority and Related Requirements

1. Contra Costa Transportation Authority/Growth Management Program/Action Plans

The Contra Costa Transportation Authority (CCTA) was originally formed to manage the funds for transportation and related infrastructure improvements generated by the half-cent sales tax enacted by the voters of Contra Costa County in 1988 under Measure C. In 2004, through Measure J, the voters extended the sales tax through the year 2034.

The CCTA carries out all implementing programs of Measures C and J, including the Growth Management Program (GMP). The CCTA further serves as the Congestion Management Agency (CMA). In such capacity, the CCTA prepares and updates the Congestion Management Program (CMP). The CMP contains several components, including: (i) traffic level of service standards for State highways and principle arterials; and (ii) multi-modal performance measures to evaluate transportation systems.

A primary component of the CCTA’s Growth Management Program is the requirement that local jurisdictions engage in “cooperative, multi-jurisdictional planning.” Such multi-jurisdictional planning in turn requires that Regional Transportation Planning Commissions (“RTMCs”) prepare “Action Plans for Routes of Regional Significance” (“Action Plans”). These Action Plans provide for “Multi-modal Transportation Service Objectives” (“MTSOs”) that establish quantifiable measures of effectiveness and include dates for attaining the stated objectives.

2. Lamorinda Action Plan Update

In the Lamorinda area, the Southwest Area Transportation Committee (“SWAT”) serves as the RTMC for purposes of preparation of an Action Plans for Lamorinda and the Tri-Valley areas. The Lamorinda Program Management Committee (LPMC) is a sub-group of SWAT for the Lamorinda area. In 1995, the LPMC adopted an Action Plan for Routes of Regional Significance, primarily focusing on State Route 24/BART corridor. In 1998, a separate Action Plan for Pleasant Hill Road was prepared. The Lamorinda Action plan was updated in

³ See City General Plan at p. II-4, and at Table 2.

2000 and most recently by Final Report prepared by DKS Associates dated December 2009 (the “Lamorinda Action Plan Update”).

The Lamorinda Action Plan Update identifies Pleasant Hill Road as a Route of Regional Significance. The MTSOs for Pleasant Hill Road set forth several objectives, including the following:

- Maintain peak hour peak direction delay index of 2.0 or lower.

The delay index (“DI”) is the ratio of the travel time during the peak hour to the travel time that would be experienced during off-peak, free-flow periods.

Abrams Associates measured the travel time conditions on Pleasant Hill Road during May 2011. Average measured test results indicated a delay index (DI) of approximately 1.7 and 1.6 during AM and PM peak hours. The average southbound AM travel time was measured to be 18.5 mph, while the northbound PM travel time was measured at 16.5 mph. The measurements confirm that existing conditions at Pleasant Hill Road are within the CCTA/MTSO standards. For the reasons set forth in this Study, the addition of Project traffic should have no significant impact on the DI and, therefore, conditions should remain consistent with applicable standards.

IV. PROJECT AND ROAD IMPROVEMENTS

A. Proposed Project

The proposed project will be a mixture of one-bedroom, two-bedroom and three-bedroom apartments, with a total of 315 units. There will also be about 9,000 square feet for the clubhouse, and a small leasing office. The project will have a total of 569 parking spaces, with 376 spaces in garages and carports, and 193 uncovered.

1. Driveway Locations and Site Access

As shown on the site plan, there are three driveways that will serve the project. At the main project entrance on Pleasant Hill Road, the applicant is proposing that a northbound left turn lane be constructed for traffic turning into the project. The proposed driveways on Deer Hill Road will each operate with all turning movements.

Driveway 1 – This is a secondary entrance on Deer Hill Road. It is located about 275 feet from the western property line. The sight distance is adequate, and this location is a good choice for access.

Driveway 2 – This is a secondary entrance on Deer Hill Road about 275 feet west of Pleasant Hill Road. Due to the combination of the downgrade and the vertical curve on Deer Hill Road, sight distance for this driveway was carefully reviewed. Field studies show that the sight distance between a vehicle on eastbound Deer Hill Road and a vehicle that is exiting the project is about 400 feet. This is sufficient to meet the standards for a 35 mph vehicle speed on Deer Hill Road. Both driveways on Deer Hill Road will provide direct access to Downtown and the BART Station without impacting any Pleasant Hill Road intersections.

Driveway 3 – This driveway is located on Pleasant Hill Road and will be the main entrance to the site. It is located about 380 feet south of Deer Hill Road. A northbound left turn lane will be constructed within the existing median on Pleasant Hill Road. This driveway location will operate safely and effectively as located. This sight distance is adequate. The driveway will result in the removal of some on-street parking spaces on Pleasant Hill Road.

The proposed northbound left turn-in-only lane is well removed from and will not affect the existing turn lane at Deer Hill Road. Preliminary engineering indicates that the new turn lane, in conjunction with the new southbound through lane, can be constructed within the existing right-of-way and a minor dedication from the Project property.

B. Project Circulation and Road Improvements

The Project includes several significant roadway, bicycle and pedestrian improvements to Pleasant Hill Road (“road and circulation improvements”), which will significantly enhance transportation conditions when the Project is built. Average vehicle delay at the Pleasant Hill and Deer Hill Road/Stanley Boulevard intersection will be reduced, even with the inclusion of Project traffic. Stated another way, if the Project is *not* built, average delay at this intersection will increase to a greater extent than if the Project *is* built. The significant Project road and circulation improvements include:

1. Project entry on Pleasant Hill Road. The Project entry includes a protected left-turn-in pocket for northbound vehicles. This improvement will keep Project traffic from having to make a left or U-turn at Deer Hill Road. Direct access to Pleasant Hill Road is designed to keep a significant amount of Project traffic from impacting the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection. The Pleasant Hill Road driveway provides direct access to the freeway without passing through any signalized intersections on Pleasant Hill Road.

2. Additional southbound lane on Pleasant Hill Road from north of Deer Hill Road to the freeway onramp. This improvement will add 50% more southbound capacity through the Pleasant Hill and Deer Hill Road/Stanley Boulevard intersection. In the AM peak hour, this additional capacity will have the added benefit of providing more pedestrian crossing time for Acalanes students and will help reduce traffic backup toward Springhill Road. During the PM peak hour, the additional southbound capacity will help increase time available for northbound left turns from Pleasant Hill Road to Deer Hill Road, which will have a positive impact on overall northbound traffic as well.

3. Sidewalk on Deer Hill Road from Pleasant Hill road to the western edge of the Property (approximately 2,000 LF). This improvement will provide pedestrian access to Acalanes High School for the neighborhood along and near Deer Hill Road approximately 3/4 miles to the west of Pleasant Hill Road.

4. Sidewalk from Deer Hill Road along the west side of Pleasant Hill Road to the existing sidewalk at the westbound freeway onramp. This sidewalk will provide direct pedestrian access from the project to Mt. Diablo Boulevard and the Downtown Area.

5. Extension of bike lane on the west side of Pleasant Hill Road from Deer Hill road, along the project frontage to the freeway. Bike lanes currently exist on the east side of Pleasant

Hill Road and on both sides of Deer Hill Road. There is no bike lane on the west side of Pleasant Hill Road, at the project frontage, but one does exist north of Deer Hill Road. The project includes extending that bike lane along the project frontage to the freeway.

C. Project Trip Generation

The trip generation calculations are shown in **Table 3**. They are based on the trip generation for Apartments (Land Use Code 220) from ITE’s Trip Generation (8th Edition).

**TABLE 3
 TRIP GENERATION CALCULATIONS**

| Land Use | ITE Code | Size | ADT | AM Peak Hour | | | PM Peak Hour | | |
|------------|----------|-----------|-------|--------------|-----|-------|--------------|-----|-------|
| | | | | In | Out | Total | In | Out | Total |
| Apartments | 220 | 315 units | 1,981 | 43 | 109 | 152 | 113 | 72 | 185 |

Based on the ITE, the trip generation rate for this apartment project is 0.59 PM peak hour trips per unit. This is consistent with the estimates of trip rates that have been made in other recent traffic studies in Walnut Creek and Pleasant Hill for similar apartment projects of this size and located roughly the same distance from BART.

The Project would have two driveways on Deer Hill Road, and one driveway on Pleasant Hill Road where these trips will enter and exit the project. The total trip generation reflects all vehicle trips that that would be generated at these three driveways. Since this Project would be all residential land uses, there were no adjustments applied to account for pass-by or internal trips. In summary, the Project is forecast to generate a total of 152 vehicle trips during the AM peak hour and about 185 trips during the PM peak hour.

The site traffic is assumed to be split between the three driveways, which will reduce the concentration of traffic at any one location. The trips generated by this proposed development are estimated for the typical peak commute hours of 8:00 to 9:00 AM and 5:00 to 6:00 PM. This represents the peak hour of “adjacent street traffic” during the time periods when the uses generally contribute to the greatest amount of congestion.

D. Project Trip Distribution

The trip distribution assumptions have been based on the project’s proximity to freeway interchanges, the existing directional split at other local driveways and intersections, and the overall land use patterns in the area. **Figure 5** presents the trip distribution percentages estimated by Abrams Associates to be used in the analysis and the AM and PM peak hour trips generated by the proposed Project at each key intersection, including the three Project driveways.

E. Project Directional Split

The percentage distribution of traffic to each of the access roadways is assumed to be as follows:

| Direction | Percent | PM Trips |
|---------------------------------|---------|----------|
| Pleasant Hill Road to the north | 13% | 24 trips |
| Deer Hill Road to the west | 18% | 33 trips |
| Stanley Boulevard to the east | 8% | 15 trips |
| Highway 24 to the west | 21% | 39 trips |
| Pleasant Hill Road to the south | 16% | 30 trips |
| Highway 24 to the east | 24% | 44 trips |

V. STUDY INTERSECTIONS AND TRAFFIC ANALYSIS SCENARIOS

The eight study intersections were evaluated based on four scenarios: Existing Conditions; Baseline Conditions; Baseline Conditions Plus Project; and Cumulative Conditions, as set forth below.

A. Scenario 1: Existing Conditions

For purposes of this Study, for Scenario 1 (Existing Conditions) Abrams Associates measured the existing intersection geometry and traffic counts at the study intersections for weekday AM and PM peak hours. The results are presented in Appendix Section 1 and are summarized below.

AM and PM peak hour turning movement counts were conducted at each of the Project study intersections in 2010 and 2011 at times when local schools were in session. **Figure 3** presents the existing lane configurations at the project study intersections and **Figure 4** presents the existing traffic volumes. **Table 4** summarizes the associated LOS computation results for the existing weekday AM and PM peak hour conditions (the corresponding LOS analysis calculation sheets are presented in the Appendix Section 1).

TABLE 4
EXISTING INTERSECTION LEVEL OF SERVICE CONDITIONS

| INTERSECTION | CONTROL | PEAK HOUR | EXISTING | |
|---|----------------|-----------|-----------------|-----|
| | | | DELAY (sec/veh) | LOS |
| 1 Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM | 3.7 | A |
| | | PM | 8.1 | A |
| 2 Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM | 5.7 | A |
| | | PM | 8.9 | A |
| 3 Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM | 12.1 | B |
| | | PM | 10.4 | B |
| 4 Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM | 9.4 | A |
| | | PM | 8.2 | A |
| 5 Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM | 26.3 | C |
| | | PM | 49.4 | D |

| | | | | | |
|---|---|-------------------|----|-------|---|
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM | 19.5 | B |
| | | | PM | 22.8 | C |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM | 10.0 | A |
| | | | PM | 11.6 | B |
| 8 | Deer Hill Road and Brown Avenue | Stop Sign Control | AM | 163.9 | F |
| | | | PM | 172.9 | F |

SOURCE: Abrams Associates, 2011

NOTE: At traffic signals, the delay is the average for all vehicles at the intersection is presented in terms of seconds per vehicle. At an unsignalized intersection, the delay is for the most critical single movement.

As shown in **Table 4**, the intersection capacity results reveal that all of the signalized intersections currently have acceptable conditions (LOS “D” or better) during the weekday AM and PM peak hours. The two-way stop on Deer Hill Road at Brown Avenue is quite different. While the overall LOS is “B” (23.2 sec), the side street movements have particularly high delay, and operate at LOS “F”. This problem could be mitigated by the use of a traffic signal.

There are seven signalized intersections on Pleasant Hill Road. The intersections at the Highway 24 ramps operate at LOS “B” and “C”, and are well below the capacity threshold. At all other intersections there is considerable excess capacity.

Of all the intersections studied, the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard is the most critical, with: (i) LOS “C” during the AM peak hour; and (ii) LOS “D” during the PM peak hour. In the AM peak hour, this intersection is affected by the traffic patterns at Acalanes High School, as well as commute traffic. In the PM peak hour, the primary factor is homeward-bound commute traffic.

To check findings in this Study, Abram Associates reviewed several City documents, including: (i) the Lafayette General Plan (2002) and its accompanying EIR; and (ii) the proposed 2009 Addendum to the Lafayette General Plan Revision Final EIR for GP02-08/RZ02-08 (the “2009 Addendum”).⁴ A review of the Lafayette General Plan in 2002; the 2009 Addendum; and the Abrams Associates “existing conditions” data prepared for this Study (in 2011), shows certain consistencies and trends in the traffic data -- specifically with regard to the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection. A review of the documents and information generally shows higher traffic at the referenced intersection in 2002; somewhat reduced traffic in 2009; and a minor increase in such traffic in 2011. (These fluctuations do not necessarily impact the corresponding LOS for the intersections for the referenced time periods.) The reasons for the fluctuations likely include the following: (i) the economy in 2002 was much more robust than during the recessionary years (including 2009), resulting in lower traffic during 2009 (and perhaps increasing during the “recovery” of 2010-2011); and (ii) minor fluctuations in traffic counts can result from the time of year data is taken;

⁴ The 2009 Addendum was prepared by Leonard Charles and Associates for the City of Lafayette as part of a proposed General Plan Amendment and Rezoning that ultimately was not adopted by the City. The 2009 Addendum was not certified or adopted by the City; however, certain background traffic data from the 2009 Addendum was reviewed by Abrams Associates to check results of this Study.

the methodology of the acquisition of data; the duration of each study and averages derived therefrom; and the interpretation/presentation of the data.

B. Scenario 2: Baseline Conditions

For purposes of this Study, for Scenario 2 (Baseline Conditions) Abrams Associates evaluated the existing traffic conditions (set forth in Scenario 1) together with the reasonably foreseeable projects in the area. The method used to project future year traffic for the baseline is based on the travel forecasts produced by the Central County ICMP computer traffic model. The County model includes trips generated in Lafayette as well as in nearby jurisdictions. The traffic projection procedures are described in detail in the Lafayette General Plan 2002 EIR.

At least two new relatively small residential projects have been identified within the study area, including the Mt. Diablo Court multi-family housing project; and a similar project on the Hungry Hunter site. These projects individually will not have significant traffic impacts; however there is expected to be continuous growth in the through traffic on Pleasant Hill Road given its regional nature. The trips added for the Baseline traffic are assumed to be at a growth rate of one-and-one-half percent per year, based on standard practice. However, based on the lack of historical growth in Lafayette and the surrounding area, and the fact that traffic actually decreased between 2002 and 2009, Abrams Associates believes that growth rate is overstated – and as a result, service levels for any scenario including this assumption is likely to be overstated.

Figure 6 shows the estimated Baseline AM and PM peak hour volumes. **Table 5**, below, summarizes the associated LOS computation results for the Baseline weekday AM and PM peak hour conditions (the corresponding LOS analysis calculation sheets are presented in Appendix Section 1). As shown in **Table 5**, all of the signalized study intersections would continue to have acceptable conditions (LOS “D” or better) during the weekday AM and PM peak hours.

**TABLE 5
 BASELINE INTERSECTION LEVEL OF SERVICE CONDITIONS**

| | INTERSECTION | CONTROL | PEAK HOUR | EXISTING | |
|---|---|----------------|-----------|-----------------|-----|
| | | | | DELAY (sec/veh) | LOS |
| 1 | Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM | 3.8 | A |
| | | | PM | 8.7 | A |
| 2 | Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM | 5.9 | A |
| | | | PM | 9.5 | A |
| 3 | Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM | 12.4 | B |
| | | | PM | 10.6 | B |
| 4 | Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM | 9.6 | A |
| | | | PM | 8.1 | A |
| 5 | Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM | 26.9 | C |
| | | | PM | 51.1 | D |
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM | 19.5 | B |
| | | | PM | 22.9 | C |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM | 9.8 | A |
| | | | PM | 11.5 | B |

| | | | | | |
|---|---------------------------------|-----------|----|-------|---|
| 8 | Deer Hill Road and Brown Avenue | Stop Sign | AM | 163.9 | F |
| | | Control | PM | 172.9 | F |

SOURCE: Abrams Associates, 2011

NOTE: At traffic signals, the delay is the average for all vehicles at the intersection, and is presented in terms of seconds per vehicle. At an unsignalized intersection (stop sign), the delay is for the most critical single movement.

The results of these baseline traffic conditions are that there are small changes in the average delay, but not enough change to affect the Levels of Service.⁵

C. Scenario 3: Baseline Conditions Plus Project Traffic

For purposes of this Study, for Scenario 3 (Baseline Conditions Plus Project Traffic) Abrams Associates added the Project-generated traffic volumes shown in **Figure 5** to the Baseline traffic volumes shown in **Figure 6**.

Figure 7 shows the Baseline Plus Project Traffic AM and PM peak hour volumes that were used in this Study to evaluate the turning movements at each study intersection.

Table 6 summarizes the LOS results for the Baseline Plus Project weekday AM and PM peak hour conditions (the corresponding LOS analysis calculation sheets are presented in the **Appendix Section 1**). **Table 6** assumes that the Project road and circulation improvements as set forth in Section IV of this Study are constructed. As shown in **Table 6**, the LOS ratings of all of the signalized study intersections would remain unaffected by the addition of Project traffic and would continue to operate at acceptable levels during AM and PM peak hours. (Project traffic would also have no effect on the LOS at Deer Hill Road and Brown Avenue since the intersection presently operates at an unsatisfactory LOS; however, the LOS could be improved with the addition of a traffic signal. At that point, the intersection would operate at an acceptable LOS.)

**TABLE 6
 BASELINE + PROJECT INTERSECTION LEVEL OF SERVICE CONDITIONS**

| INTERSECTION | CONTROL | PEAK HOUR | EXISTING | |
|--|----------------|-----------|-----------------|-----|
| | | | DELAY (sec/veh) | LOS |
| 1 Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM | 3.7 | A |
| | | PM | 8.8 | A |
| 2 Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM | 6.0 | A |
| | | PM | 9.5 | A |
| 3 Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM | 12.3 | B |
| | | PM | 10.6 | B |
| 4 Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM | 9.7 | A |
| | | PM | 8.1 | A |

⁵ The reader will note that in some cases in each of these Tables the Delay may decrease by a small amount (e.g., by .10) even with a small amount of additional assumed traffic. This anomaly occurs where the underlying model automatically shifts slightly.

| | | | | | |
|---|---|-------------------|----|-------|---|
| 5 | Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM | 23.7 | C |
| | | | PM | 52.0 | D |
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM | 19.5 | B |
| | | | PM | 22.8 | C |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM | 9.7 | A |
| | | | PM | 11.4 | B |
| 8 | Deer Hill Road and Brown Avenue | Stop Sign Control | AM | 182.5 | F |
| | | | PM | 202.5 | F |

SOURCE: Abrams Associates, 2011

NOTE: At traffic signals, the delay is the average for all vehicles at the intersection is presented in terms of seconds per vehicle. At an unsignalized intersection, the delay is for the most critical single movement.

1. Conditions with Pedestrian Actuation

The City of Lafayette has requested that as part of this Study the AM peak hour intersection capacity conditions at Deer Hill Road be tested with the condition that there would be a pedestrian actuation during each signal phase. This would apply for the AM peak hour during the period when there is maximum activity in the vicinity of Acalanes High School. The results of this analysis are that the intersection will operate at LOS “C” with an average delay of 26.9 seconds per vehicle. (This is in comparison to the LOS “C”, average vehicle delay of 23.7 seconds, which occurs with the normal calculations. It is more likely that there would be pedestrian actuation for only a few cycles during the AM peak hour which is the assumption which was used throughout this Study).

2. Roadway and Intersection Mitigation Measures

With the addition of Project traffic and the proposed Project road and circulation improvements along Pleasant Hill Road, all of the signalized intersection delay results are similar or less than without the Project, and meet the City of Lafayette and CCTA standards. The Pleasant Hill Road and Deer Hill/Stanley intersection will experience a significant (12%) reduction in delay during the AM peak hour, with the proposed project improvements. Therefore, no mitigation measures are required as a result of Project traffic.

3. Deer Hill Road/Brown Avenue Intersection

The Terraces project will add additional traffic volumes to this intersection, as much as 33 vehicles per hour (total both directions) during the PM peak hour. This intersection is currently a two-way stop with the stops on Brown Avenue, and *could* meet the warrants for the installation of a traffic signal. However, there are a number of competing factors at this location. The traffic signal could provide some speed control for speeding cars on Deer Hill Road. Should a traffic signal be installed by the City of Lafayette, the project should make a fair share contribution.

4. Internal Circulation and Access

Abrams Associates has worked with the site planner to layout an effective internal circulation system. No internal site circulation or access issues have been identified that would

cause a traffic safety problem or any unusual traffic congestion or delay. It should be noted that the volumes on the internal roadways would be light enough so that no significant conflicts would be expected with vehicles backing out of the garages and/or parking spaces within the project.

5. Capacity Impacts at Project Driveways

Table 7 shows the results of the intersection capacity studies at the three driveways that serve the project. On Pleasant Hill Road, there will be a left turn lane for entering traffic, but the outbound left turn would be prohibited. The LOS is calculated to be “C” under this condition. There are more than enough gaps in the through traffic so the movement can operate without delay.

TABLE 7
LEVEL OF SERVICE CONDITIONS AT PROJECT DRIVEWAYS
(Existing plus project traffic)

| | INTERSECTION | CONTROL | PEAK HOUR | Existing + Project | |
|---|---|-------------------|-----------|--------------------|-----|
| | | | | DELAY (sec/veh) | LOS |
| 1 | Project Entrance on Pleasant Hill Road | Stop Sign Control | AM | 14.5 | B |
| | | | PM | 10.9 | B |
| 2 | Project Driveway (East) on Deer Hill Road | Stop Sign Control | AM | 18.1 | C |
| | | | PM | 21.2 | C |
| 3 | Project Driveway (West) on Deer Hill Road (Top of the Hill) | Stop Sign Control | AM | 13.6 | C |
| | | | PM | 18.2 | C |

The project driveways on Deer Hill Road will all operate at acceptable Levels of Service and will not cause any capacity or safety issues on the major street. At the Pleasant Hill Road driveway, the inbound left turn results in a significant improvement at Deer Hill Road by removing potential left turns and U-turns.

6. Parking

The proposed project would provide an adequate supply of off-street parking based on the City’s requirements. A total of 569 parking spaces are provided which equates to a parking ratio of 1.81 spaces per unit. This is consistent with other nearby projects and meets the City zoning standards.

D. Scenario 4: Cumulative Conditions

For purposes of this Study, for Scenario 4 (Cumulative Conditions) Abrams Associates evaluated the intersection traffic volumes based on the existing turning movements plus the addition of growth forecast by the County’s traffic model. For this analysis the 2030 cumulative traffic volumes were developed by applying a 0.55%/year increase to the background traffic volumes. The cumulative analyses below include results: (i) without the Project; and (ii) with the Project and its traffic and circulation improvements (using HCM and CCTA methodology).

1. Cumulative without Project

Figure 8 shows the cumulative AM and PM peak hour traffic volumes at each of the project study intersections without the Project.

Table 8 summarizes the associated LOS computation results for the cumulative (year 2030) weekday AM and PM peak hour traffic conditions without the Project. The corresponding LOS analysis calculation sheets are presented in **Appendix Section 1**.

As shown in **Table 8**, all of the signalized study intersections would continue to have acceptable conditions (LOS “D” or better) during the weekday AM and PM peak hours, with the exception of Pleasant Hill Road and Deer Hill Road/Stanley Boulevard during the PM peak hour.

TABLE 8
CUMULATIVE (2030) INTERSECTION LEVEL OF SERVICE CONDITIONS
(without Project)

| | INTERSECTION | CONTROL | PEAK HOUR | CUMULATIVE (2030) | |
|---|---|-------------------|-----------|-------------------|--------|
| | | | | DELAY (sec/veh) | LOS |
| 1 | Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM PM | 4.1 12.1 | A B |
| 2 | Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM PM | 6.6 14.1 | A B |
| 3 | Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM PM | 15.0 11.8 | B B |
| 4 | Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM PM | 12.6 8.3 | B A |
| 5 | Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM PM | 32.1 71.1 | C E |
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM PM | 20.2 24.0 | C C |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM PM | 10.2 12.2 | B B |
| 8 | Deer Hill Road and Brown Avenue | Stop Sign Control | AM PM | 295.3 353.4 | F F |

SOURCE: Abrams Associates, 2011

NOTE: At traffic signals, the delay is the average for all vehicles at the intersection is presented in terms of seconds per vehicle. At an unsignalized intersection, the delay is for the most critical single movement.

2. Cumulative with the Project

Table 9 summarizes the associated LOS computation results for the Cumulative (Year 2030) weekday AM and PM peak hour traffic conditions with the Project and its traffic and circulation improvements. The corresponding LOS analysis calculation sheets are presented in the **Appendix Section 1**.

As shown in **Table 9**, all of the signalized study intersections would continue to have acceptable conditions (LOS “D” or better) during the weekday AM and PM peak hours, again with the exception of Pleasant Hill Road and Deer Hill Road/Stanley Boulevard during the PM peak hour. However, the Project will not contribute to such degradation of level of service. The Project, with its traffic and circulation improvements, will improve traffic and circulation at this intersection, and will lessen the severity of any future degradation of Level of Service based on future, cumulative traffic.

**TABLE 9
 CUMULATIVE (2030) LEVEL OF SERVICE CONDITIONS (with Project)**

| | INTERSECTION | CONTROL | PEAK HOUR | CUMULATIVE (2030) | |
|---|---|-------------------|-----------|-------------------|-----|
| | | | | DELAY (sec/veh) | LOS |
| 1 | Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM | 4.0 | A |
| | | | PM | 12.2 | B |
| 2 | Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM | 6.6 | A |
| | | | PM | 14.2 | B |
| 3 | Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM | 15.0 | B |
| | | | PM | 11.9 | B |
| 4 | Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM | 12.6 | B |
| | | | PM | 8.3 | A |
| 5 | Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM | 30.1 | C |
| | | | PM | 69.4 | E |
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM | 20.2 | C |
| | | | PM | 24.0 | C |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM | 10.1 | B |
| | | | PM | 12.2 | B |
| 8 | Deer Hill Road and Brown Avenue | Stop Sign Control | AM | 324.3 | F |
| | | | PM | 406.5 | F |

3. Cumulative Capacity Using the CCTA Methodology

Table 10 shows the results of the intersection Capacity Calculations using the CCTA calculation procedures. This methodology is based on the results being reported in terms of volume to capacity ratio (v/c ratio), and generally shows less critical results than the HCM Methodology. (See **Appendix Section 2.**)

**TABLE 10
 CUMULATIVE (2030) LEVEL OF SERVICE CONDITIONS WITH THE PROJECT
 Using the CCTA Methodology**

| | INTERSECTION | CONTROL | PEAK HOUR | CUMULATIVE | |
|---|---|----------------|-----------|--------------|-----|
| | | | | Vol/Capacity | LOS |
| 1 | Pleasant Hill Road and Rancho View Drive | Traffic Signal | AM | .486 | A |
| | | | PM | .674 | B |
| 2 | Pleasant Hill Road and Green Valley Drive | Traffic Signal | AM | .528 | A |
| | | | PM | .664 | B |

| | | | | | |
|---|---|-------------------|----------|------|-----|
| 3 | Pleasant Hill Road and Reliez Valley Road | Traffic Signal | AM | .619 | B |
| | | | PM | .659 | B |
| 4 | Pleasant Hill Road and Springhill Road/Quandt Road | Traffic Signal | AM | .741 | C |
| | | | PM | .738 | C |
| 5 | Pleasant Hill Road and Deer Hill Road/Stanley Boulevard | Traffic Signal | AM | .725 | C |
| | | | PM | .913 | E |
| 6 | Pleasant Hill Road and Mt Diablo Blvd/EB Hwy 24 On-ramp | Traffic Signal | AM | .457 | A |
| | | | PM | .603 | B |
| 7 | Pleasant Hill Road and EB Hwy 24 Off-ramp/Old Tunnel Road | Traffic Signal | AM | .552 | A |
| | | | PM | .678 | B |
| 8 | Deer Hill Road and Brown Avenue | Stop Sign Control | AM PM | N/A | N/A |

SOURCE: Abrams Associates, 2011

NOTE: At traffic signals, the results are presented in terms of volume to capacity ratios. This methodology is not applicable to unsignalized intersections.

The data on cumulative impacts shows that the cumulative impacts of development in the area to the year 2030 will continue to degrade the Level of Service of the intersection of Pleasant Hill Road and Deer Hill Road/Stanley Boulevard. However, the Project will not contribute to such degradation of level of service. The Project, with its traffic and circulation improvements, will improve traffic and circulation at this intersection, and will lessen the severity of any future degradation of Level of Service based on future, cumulative traffic.

4. Deer Hill Road and Brown Avenue

As set forth in this Study, the Deer Hill Road and Brown Avenue intersection, which is unsignalized, operates at LOS "F" under all Scenarios (Existing through Cumulative conditions). This condition will continue with or without the Project and the Project's traffic and circulation improvements. This condition could be addressed with the construction of a traffic signal.

5. State Highway 24

The addition of traffic to State Highway 24 is problematic. State Highway 24 is impacted by regional traffic and regulated in all aspects by the State of California. The addition of Project traffic to State Highway 24 cannot be deemed to have a significant adverse impact because State Highway 24 collects and transports traffic to and from so many different jurisdictions that it is impossible for any individual project - or even jurisdiction - to effectively mitigate traffic impacts. Essentially, existing and future conditions of traffic on State Highway 24 will not be impacted in any measurable or meaningful way by traffic generated to or from this Project.

VI. FINDINGS AND CONCLUSIONS

While a total of eight (8) intersections and several roadways are analyzed in this Study, it is clear that the area with the greatest potential for impacts from Project traffic is along Pleasant Hill Road, and specifically the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard

intersection. Currently, the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection operates at an acceptable Level of Service during both the AM and PM peak hours. However, there is a concern regarding future traffic travelling southbound through the intersection during the AM peak hour, and northbound through the intersection during the PM peak hour. Without the Project, such increasing traffic (from future build-out within the City) is expected to eventually degrade the referenced Level of Service.

With the construction of the Project's proposed road and circulation improvements, the additional Project traffic will not have a significant impact on existing traffic levels and the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard intersection.

The Project, with the proposed road and circulation improvements, will significantly improve the traffic conditions along Pleasant Hill Road and at the Pleasant Hill Road and Deer Hill Road/Stanley Boulevard Intersection. This is primarily due to the construction of the additional southbound through-lane along Pleasant Hill Road.

The Project, with the proposed road and circulation improvements, is consistent with the standards set by the Contra Costa Transportation Authority, and specifically the Multi-modal Transportation Service Objectives (MTSOs) as listed in the Lamorinda Action Plan and the Pleasant Hill Road Action Plan.

The data on cumulative impacts shows that the cumulative impacts of development in the area to the year 2030 will continue to degrade the level of service of the intersection of Pleasant Hill Road and Deer Hill Road/Stanley Boulevard. However, the Project will not contribute to such degradation of level of service. The Project, with its road and circulation improvements, will improve traffic and circulation at this intersection, and will lessen the severity of any future degradation of level of service based on future, cumulative traffic.

The Deer Hill Road and Brown Avenue intersection, which is unsignalized, operates at LOS "F" under all Scenarios (Existing through Cumulative conditions). This condition will continue with or without the Project and the Project's traffic and circulation improvements. This condition could be addressed with the construction of a traffic signal.

State Highway 24 is impacted by regional traffic and regulated in all aspects by the State of California. The addition of Project traffic to State Highway 24 cannot be deemed to have a significant adverse impact because State Highway 24 collects and transports traffic to and from so many different jurisdictions that it is impossible for any individual project - or even jurisdiction - to effectively mitigate traffic impacts. Essentially, existing and future conditions of traffic on State Highway 24 will not be impacted in any measurable or meaningful way by traffic generated to or from this Project.

For these and other reasons set forth in this Study, the Project traffic will not have a significant adverse effect or impact on existing and future traffic conditions.

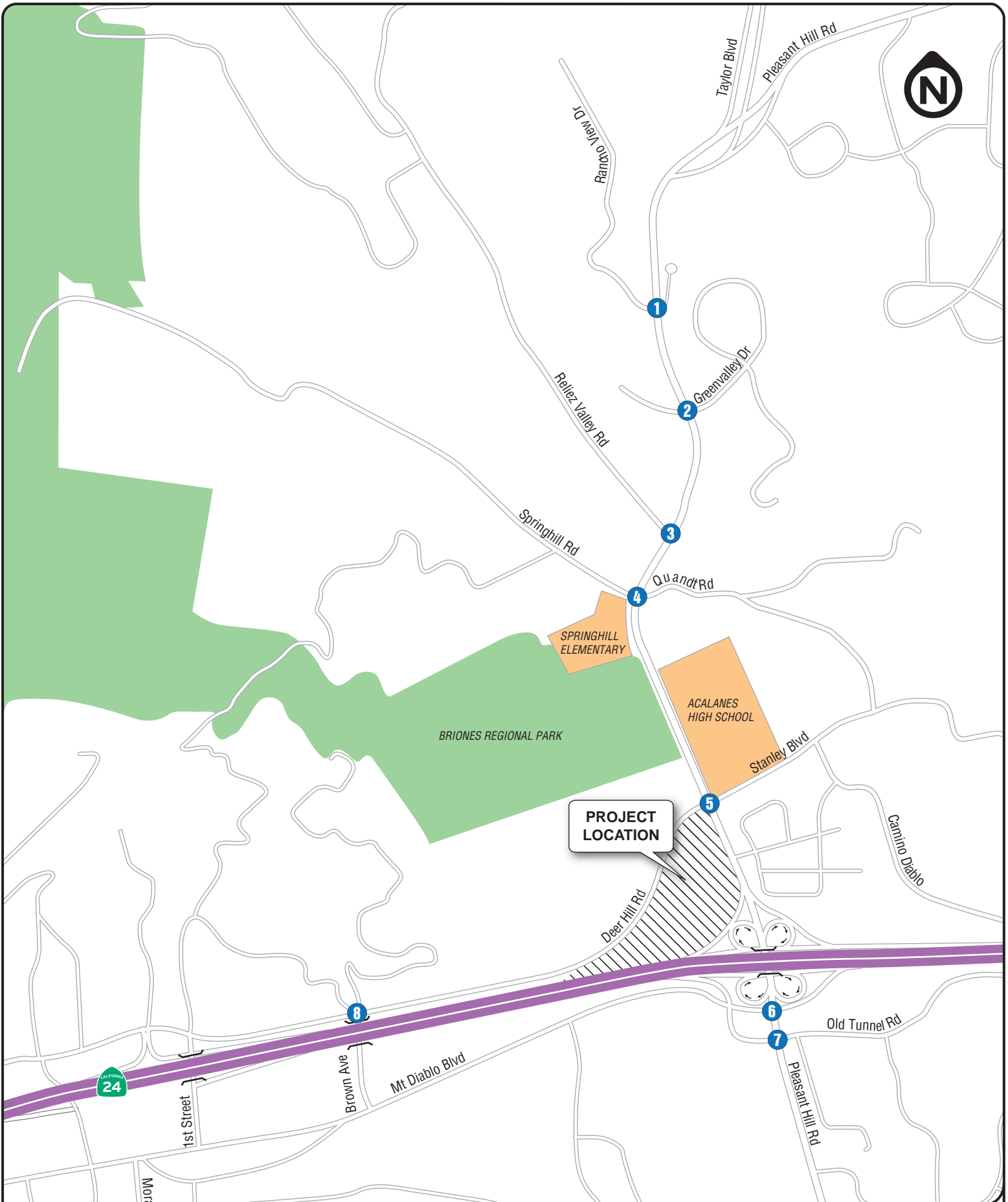
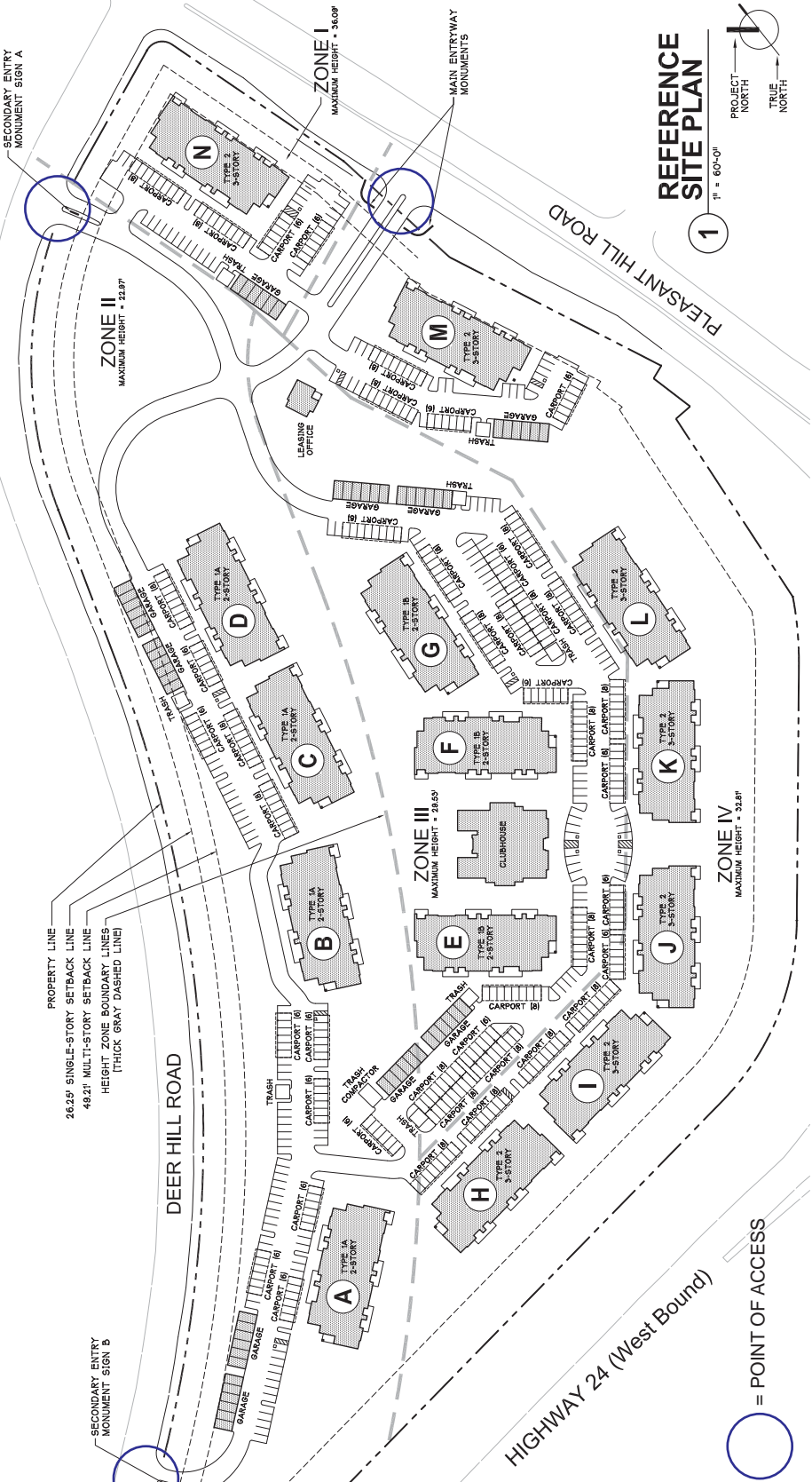


FIGURE 1 | PROJECT LOCATION
TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette



SCALE: 1" = 60'-0"
 DATE: 3/21/2011
 REVISIONS:
 REFERENCE
 SITE PLAN

| | |
|-------------|-------|
| PROJECT NO. | 10050 |
| SHEET | OF |



| PROJECT DATA | | BUILDING DATA & UNIT COUNTS | | TOTAL | |
|--|---|-----------------------------|---------------------|----------------------------|-------------------------|
| SITE AREA | 970,272 SF (22.27 ACRES) | 1ST FLOOR AREA (SF) | 2ND FLOOR AREA (SF) | NO. OF UNITS | MAX. HEIGHT (FEET) |
| TOTAL BUILDING FOOTPRINT | 2,048,832 SF (INCLUDING ACCESSORY STRUCTURES) | 9,425 | 9,545 | 10 | 22.97' |
| COVERAGE | 211% | 9,425 | 9,545 | 26 | 22.97' |
| NUMBER OF UNITS | 315 | 9,425 | 9,545 | 8 | 22.97' |
| DENSITY | 14.1 UNITS/ACRES | 9,425 | 9,545 | 8 | 22.97' |
| PARKING | | 9,425 | 9,545 | 1 | 28.59' |
| RESIDENTIAL PARKING RATIO | 1.62 PER UNIT | 9,425 | 9,545 | 1 | 28.59' |
| 1-BDR | 140 | 9,425 | 9,545 | 1 | 32.81' |
| 2-BDR | 168 | 9,425 | 9,545 | 1 | 32.81' |
| 3-BDR | 200 | 9,425 | 9,545 | 1 | 32.81' |
| GUEST | 63 | 9,425 | 9,545 | 1 | 36.09' |
| TOTAL | 511 STALLS REQUIRED (BASED ON M-F-B) | 13,850 | 13,850 | 1 | 36.09' |
| PARKING SUMMARY | | 13,850 | 13,850 | 1 | 36.09' |
| GARAGES | 60 | 13,850 | 13,850 | 1 | 36.09' |
| UNCOVERED | 186 | 13,850 | 13,850 | 1 | 36.09' |
| TOTAL | 246 | 13,850 | 13,850 | 1 | 36.09' |
| RATIO | 1.81 PER UNIT | 13,850 | 13,850 | 1 | 36.09' |
| BUILDING DATA & UNIT COUNTS | | 13,850 | 13,850 | 1 | 36.09' |
| DESCRIPTION | 1ST FLOOR AREA (SF) | 2ND FLOOR AREA (SF) | NO. OF UNITS | MAX. HEIGHT (FEET) | |
| APARTMENT A | 9,425 | 9,545 | 10 | 22.97' | |
| APARTMENT B | 9,425 | 9,545 | 26 | 22.97' | |
| APARTMENT C | 9,425 | 9,545 | 8 | 22.97' | |
| APARTMENT D | 9,425 | 9,545 | 8 | 22.97' | |
| APARTMENT E | 9,425 | 9,545 | 1 | 28.59' | |
| APARTMENT F | 9,425 | 9,545 | 1 | 28.59' | |
| APARTMENT G | 9,425 | 9,545 | 1 | 28.59' | |
| APARTMENT H | 9,425 | 9,545 | 1 | 32.81' | |
| APARTMENT I | 9,425 | 9,545 | 1 | 32.81' | |
| APARTMENT J | 9,425 | 9,545 | 1 | 32.81' | |
| APARTMENT K | 9,425 | 9,545 | 1 | 32.81' | |
| APARTMENT L | 9,425 | 9,545 | 1 | 36.09' | |
| APARTMENT M | 9,425 | 9,545 | 1 | 36.09' | |
| APARTMENT N | 9,425 | 9,545 | 1 | 36.09' | |
| SUBTOTAL | 13,850 | 13,850 | 66,815 | 332,385 SF | |
| CLUBHOUSE | 800 | 5,270 | 13,300 | 28.59' | |
| LEASING OFFICE | 950 | 950 | 950 | 28.59' | |
| TOTAL | 140,850 | 138,800 | 66,815 | 316,645 SF | |
| BUILDING DATA & UNIT COUNTS | | 140,850 | 138,800 | 66,815 | 316,645 SF |
| DESCRIPTION | NUMBER | AREA (SF) | TOTAL AREA (SF) | TOTAL UNITS PER BLDG. TYPE | TOTAL UNITS FOR PROJECT |
| GARAGES | 10 | 1,300 | 13,300 | 2 | 18 |
| 8-CAR CARPORTS | 26 | 1,224 | 3,184 | 8 | 18 |
| TRASH COMPACTOR | 1 | 237 | 237 | 2 | 27 |
| TRASH ENCLOSURE | 8 | 358 | 358 | 3 | 35 |
| TOTAL | 45 | 3,119 | 63,802 SF | 15 | 100.0% |
| BUILDING TYPE | STORIES | 1-BDR | 2-BDR | 3-BDR | |
| 1A | TWO | 8 | 8 | 2 | 18 |
| 1B | TWO | 8 | 8 | 2 | 18 |
| 2 | THREE | 12 | 12 | 3 | 27 |
| BUILDING TYPE | AMOUNT | 1-BDR | 2-BDR | 3-BDR | |
| 1A | 4 | 32 | 8 | --- | --- |
| 1B | 7 | 84 | 21 | --- | --- |
| 2 | 2 | 140 | 35 | --- | --- |
| TOTALS | PROJECT MIX | 44.4% | 44.4% | 44.4% | 111% |

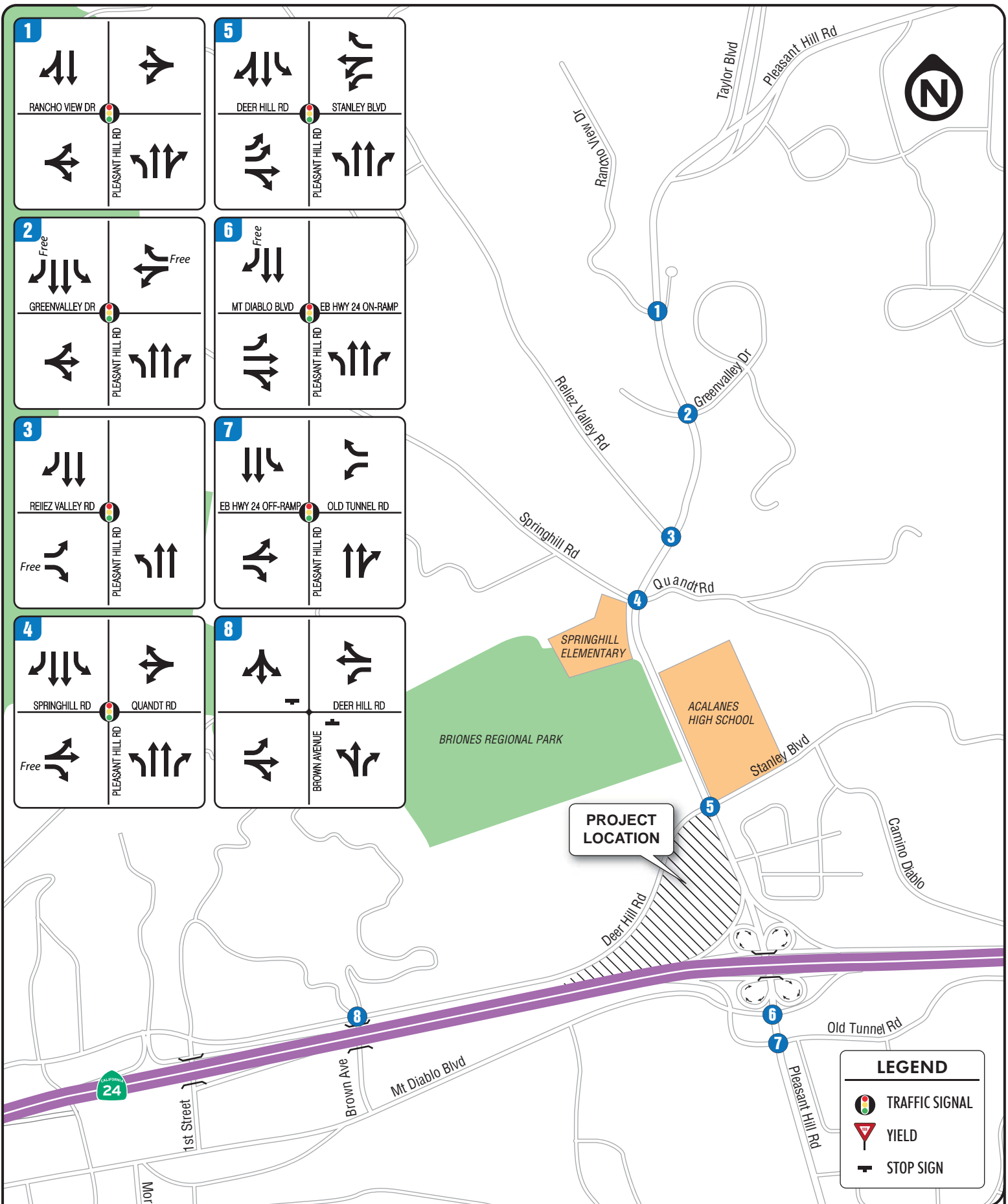


FIGURE 3 | EXISTING LANE CONFIGURATIONS
TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette

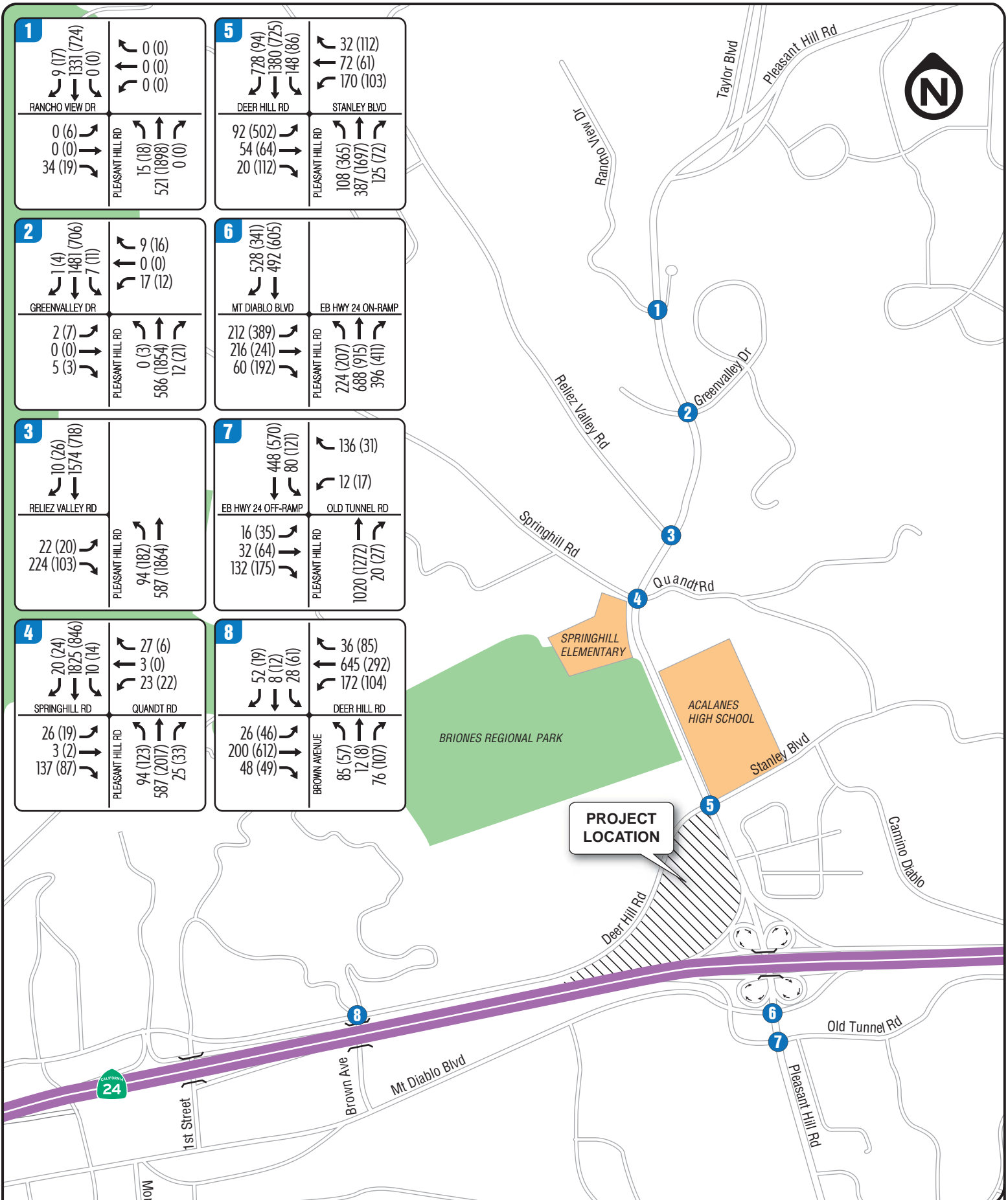


FIGURE 4 | EXISTING AM (PM) PEAK HOUR VOLUMES
 TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette

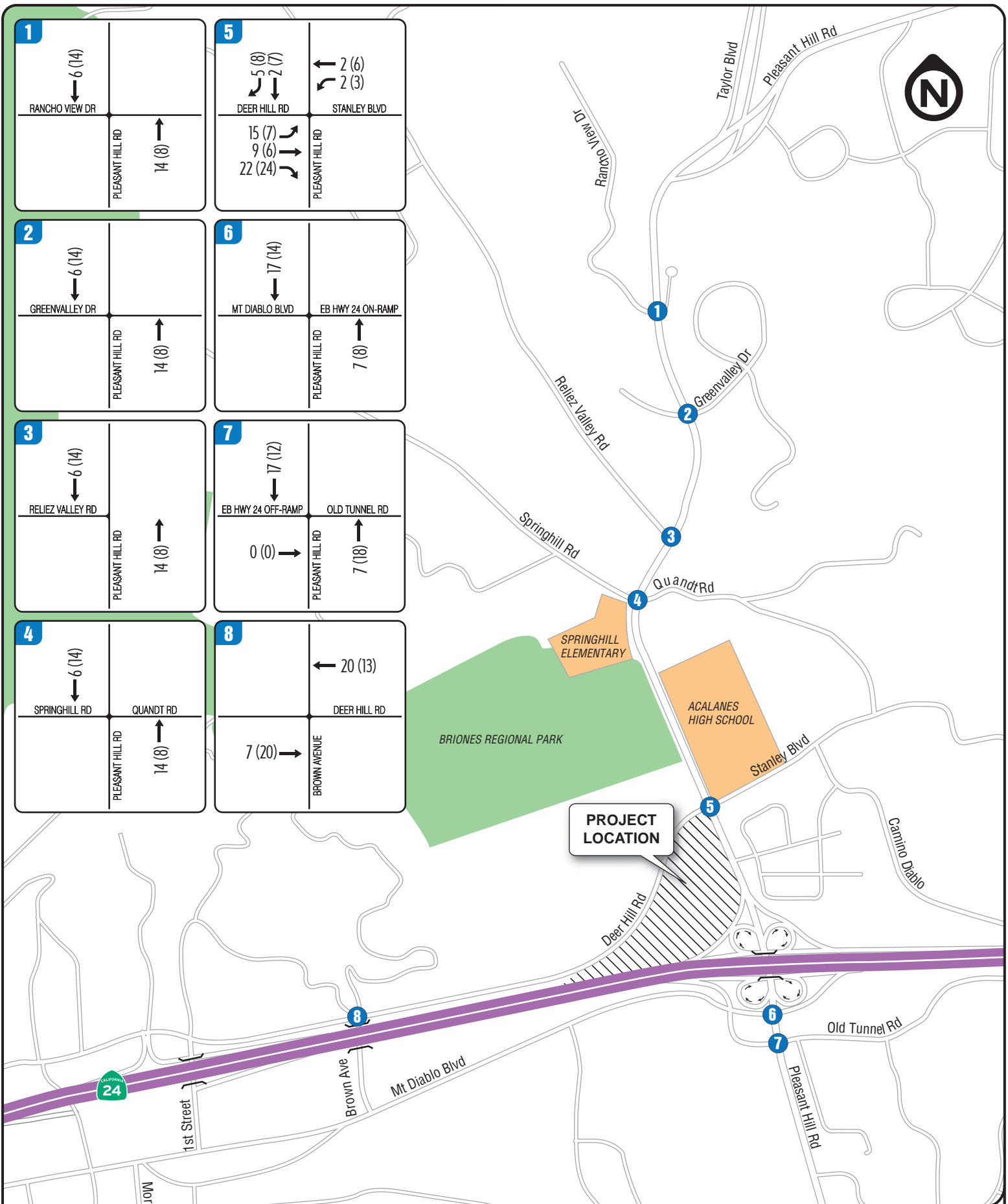


FIGURE 5 | PROJECT TRIP GENERATION
TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette

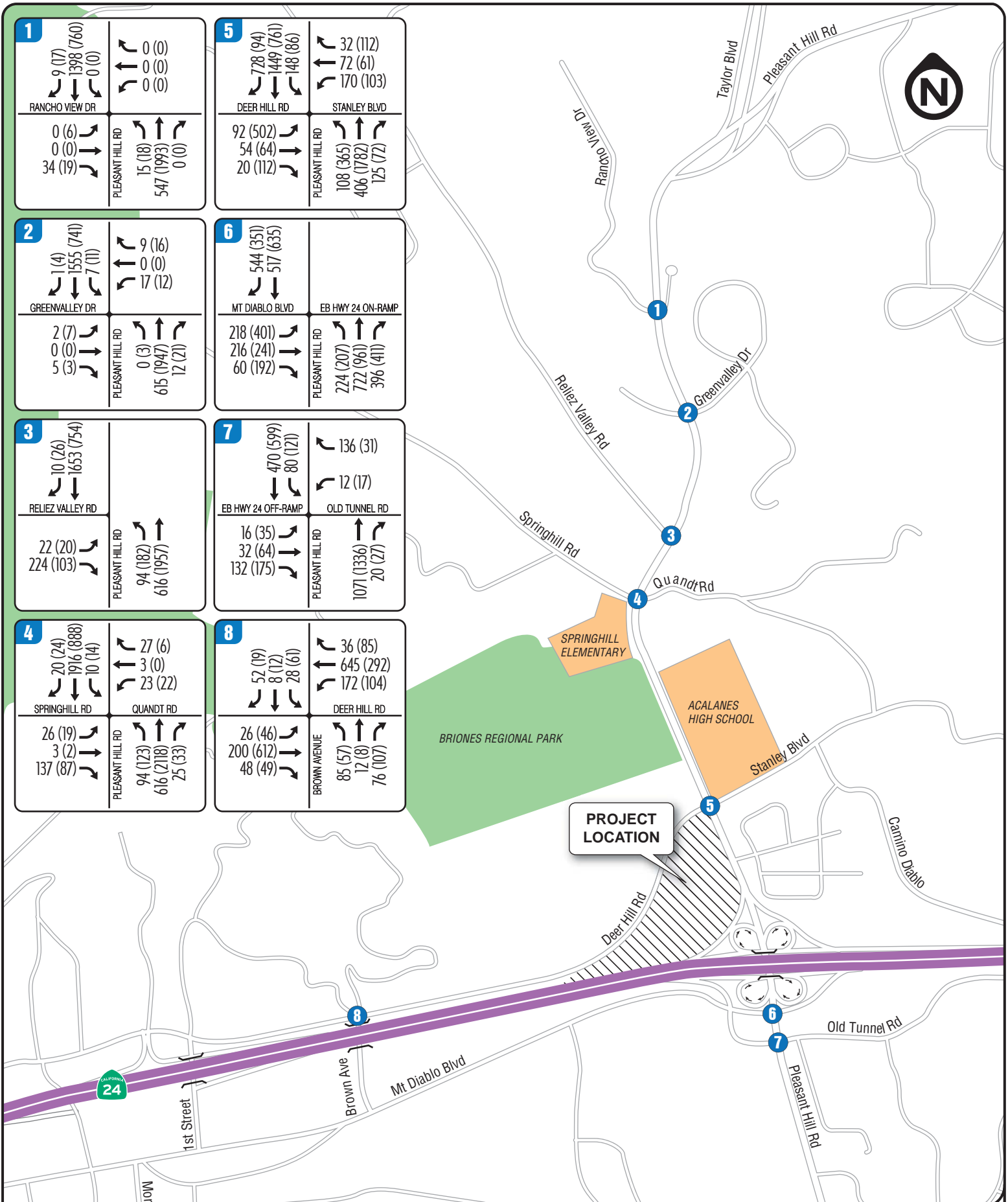


FIGURE 6 | BASELINE AM (PM) PEAK HOUR VOLUMES
TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette

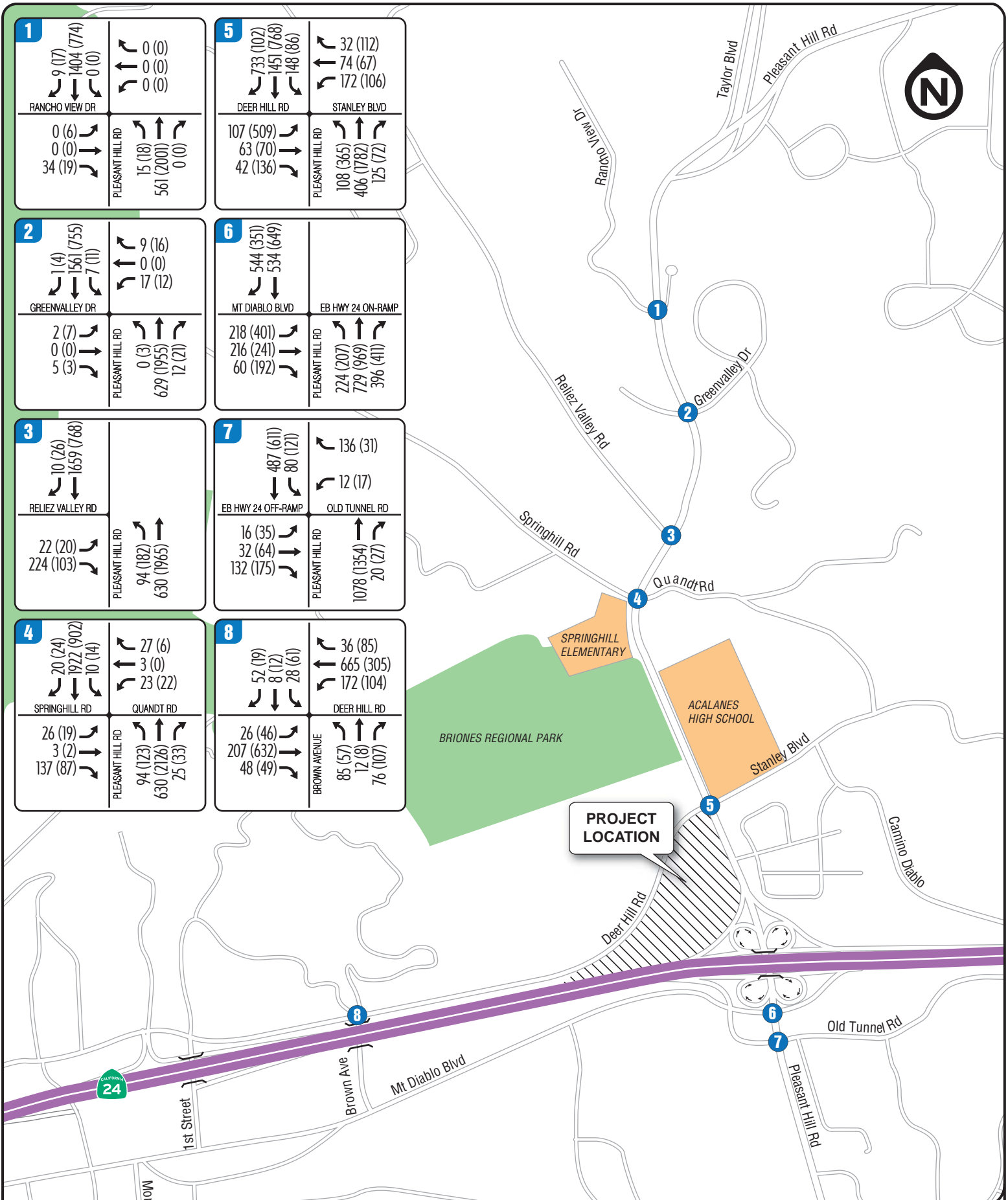


FIGURE 7 | BASELINE PLUS PROJECT AM (PM) PEAK HOUR VOLUMES

TRAFFIC IMPACT STUDY

The Terraces of Lafayette
 City of Lafayette

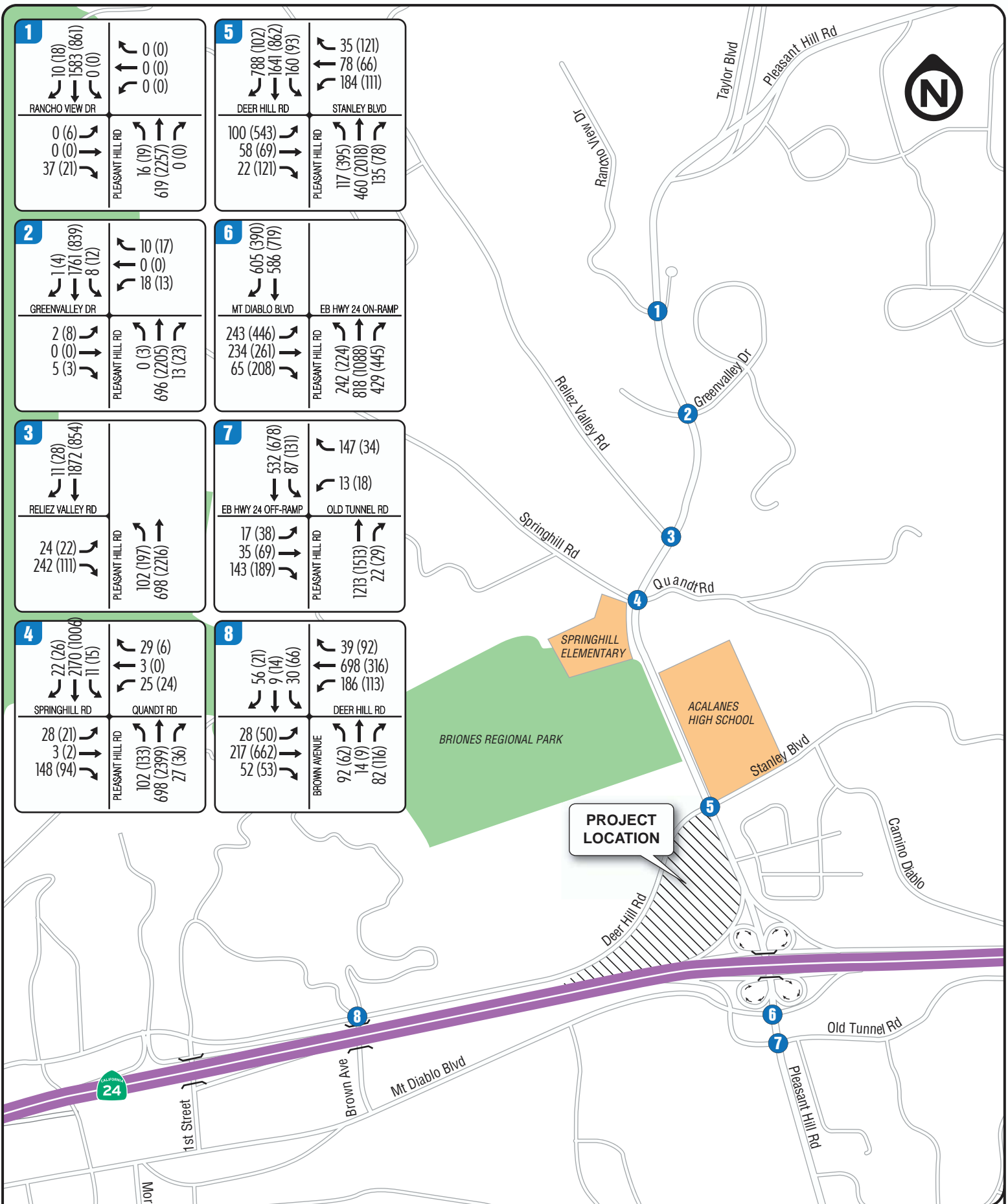


FIGURE 8 | CUMULATIVE AM (PM) PEAK HOUR VOLUMES
TRAFFIC IMPACT STUDY
 The Terraces of Lafayette
 City of Lafayette

TECHNICAL APPENDIX

THE TERRACES OF LAFAYETTE

Prepared by:
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Walnut Creek, CA 94596
Tel: 925.945.0201



Abrams Associates
TRAFFIC ENGINEERING, INC.

June, 2011

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|-------|------|------|-------|------|------|------|------|
| Lane Configurations | | ↔ | | | ↔ | | | ↔ | | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | | 4.0 | | | 4.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 1.00 | | | 0.95 | | | 0.95 | | |
| Frt | 0.86 | | | 1.00 | | | 1.00 | | | 1.00 | | |
| Flt Protected | 1.00 | | | 0.95 | | | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1611 | | | 1770 | | | 3539 | | | 3536 | | |
| Flt Permitted | 1.00 | | | 0.95 | | | 1.00 | | | 1.00 | | |
| Satd. Flow (perm) | 1611 | | | 1770 | | | 3539 | | | 3536 | | |
| Volume (vph) | 0 | 0 | 34 | 0 | 0 | 0 | 15 | 521 | 0 | 0 | 1331 | 9 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 37 | 0 | 0 | 0 | 16 | 566 | 0 | 0 | 1447 | 10 |
| RTOR Reduction (vph) | 0 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 1 | 0 | 0 | 0 | 0 | 16 | 566 | 0 | 0 | 1457 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | | |
| Permitted Phases | | | | | | | | | | | | |
| Actuated Green, G (s) | 3.3 | | | | 3.1 | | 98.7 | 91.6 | | | | |
| Effective Green, g (s) | 3.3 | | | | 3.1 | | 98.7 | 91.6 | | | | |
| Actuated g/C Ratio | 0.03 | | | | 0.03 | | 0.90 | 0.83 | | | | |
| Clearance Time (s) | 4.0 | | | | 4.0 | | 4.0 | 4.0 | | | | |
| Vehicle Extension (s) | 3.0 | | | | 3.0 | | 3.0 | 3.0 | | | | |
| Lane Grp Cap (vph) | 48 | | | | 50 | | 3175 | 2945 | | | | |
| v/s Ratio Prot | c0.02 | | | | c0.01 | | 0.16 | c0.41 | | | | |
| v/s Ratio Perm | | | | | | | | | | | | |
| v/c Ratio | 0.02 | | | | 0.32 | | 0.18 | 0.49 | | | | |
| Uniform Delay, d1 | 51.8 | | | | 52.4 | | 0.7 | 2.6 | | | | |
| Progression Factor | 1.00 | | | | 0.81 | | 0.73 | 1.00 | | | | |
| Incremental Delay, d2 | 0.2 | | | | 3.6 | | 0.1 | 0.6 | | | | |
| Delay (s) | 52.0 | | | | 46.3 | | 0.6 | 3.2 | | | | |
| Level of Service | D | | | | D | | A | A | | | | |
| Approach Delay (s) | 52.0 | | 0.0 | | 1.9 | | 3.2 | | | | | |
| Approach LOS | D | | A | | A | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 3.7 | | HCM Level of Service | | A | | | | | | | |
| HCM Volume to Capacity ratio | 0.50 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 47.1% | | ICU Level of Service | | A | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|-------|------|------|------|------|------|-------|------|
| Lane Configurations | | ↔ | | | ↔ | | | ↔ | | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | | 4.0 | | | 4.0 | | | 4.0 | | |
| Lane Util. Factor | 1.00 | | | 1.00 | | | 0.95 | | | 0.95 | | |
| Frt | 0.90 | | | 1.00 | | | 0.85 | | | 1.00 | | |
| Flt Protected | 0.99 | | | 0.95 | | | 1.00 | | | 1.00 | | |
| Satd. Flow (prot) | 1659 | | | 1770 | | | 1583 | | | 3539 | | |
| Flt Permitted | 0.96 | | | 0.75 | | | 1.00 | | | 1.00 | | |
| Satd. Flow (perm) | 1618 | | | 1403 | | | 1583 | | | 3539 | | |
| Volume (vph) | 2 | 0 | 5 | 17 | 0 | 9 | 0 | 586 | 12 | 7 | 1481 | 1 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 2 | 0 | 5 | 18 | 0 | 10 | 0 | 637 | 13 | 8 | 1610 | 1 |
| RTOR Reduction (vph) | 0 | 4 | 0 | 0 | 0 | 8 | 0 | 0 | 4 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 3 | 0 | 0 | 18 | 2 | 0 | 637 | 9 | 8 | 1610 | 1 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 16.8 | | | | 16.8 | | 16.8 | 80.0 | | 80.0 | 1.2 | 85.2 |
| Effective Green, g (s) | 16.8 | | | | 16.8 | | 16.8 | 80.0 | | 80.0 | 1.2 | 85.2 |
| Actuated g/C Ratio | 0.15 | | | | 0.15 | | 0.15 | 0.73 | | 0.73 | 0.01 | 0.77 |
| Clearance Time (s) | 4.0 | | | | 4.0 | | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | | | | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 247 | | | | 214 | | 242 | 2574 | | 1151 | 19 | 2741 |
| v/s Ratio Prot | 0.00 | | | | c0.01 | | 0.01 | 0.18 | | 0.00 | c0.45 | |
| v/s Ratio Perm | 0.01 | | | | 0.08 | | 0.01 | 0.25 | | 0.01 | 0.42 | 0.59 |
| v/c Ratio | 0.01 | | | | 0.08 | | 0.01 | 0.25 | | 0.01 | 0.42 | 0.59 |
| Uniform Delay, d1 | 39.6 | | | | 40.0 | | 39.5 | 5.0 | | 4.1 | 54.1 | 5.1 |
| Progression Factor | 1.00 | | | | 1.00 | | 1.00 | 1.24 | | 1.84 | 1.23 | 0.68 |
| Incremental Delay, d2 | 0.0 | | | | 0.2 | | 0.0 | 0.2 | | 0.0 | 13.3 | 0.9 |
| Delay (s) | 39.6 | | | | 40.2 | | 39.5 | 6.4 | | 7.6 | 79.7 | 4.4 |
| Level of Service | D | | | | D | | D | A | | A | E | A |
| Approach Delay (s) | 39.6 | | 39.9 | | 6.4 | | 4.7 | | | | | |
| Approach LOS | D | | D | | A | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 5.7 | | HCM Level of Service | | A | | | | | | | |
| HCM Volume to Capacity ratio | 0.50 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | | | | | | | |
| Intersection Capacity Utilization | 50.9% | | ICU Level of Service | | A | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|------|----------------------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 22 | 224 | 94 | 587 | 1574 | 10 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 24 | 243 | 102 | 638 | 1711 | 11 |
| RTOR Reduction (vph) | 0 | 180 | 0 | 0 | 0 | 3 |
| Lane Group Flow (vph) | 24 | 63 | 102 | 638 | 1711 | 8 |
| Turn Type | Perm | Perm | Prot | | Perm | |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | | 4 | | | | 6 |
| Actuated Green, G (s) | 10.1 | 10.1 | 11.2 | 91.9 | 76.7 | 76.7 |
| Effective Green, g (s) | 10.1 | 10.1 | 11.2 | 91.9 | 76.7 | 76.7 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.10 | 0.84 | 0.70 | 0.70 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 163 | 145 | 180 | 2957 | 2468 | 1104 |
| v/s Ratio Prot | 0.01 | | c0.06 | 0.18 | c0.48 | |
| v/s Ratio Perm | | 0.15 | | | | 0.01 |
| v/c Ratio | 0.15 | 0.44 | 0.57 | 0.22 | 0.69 | 0.01 |
| Uniform Delay, d1 | 46.0 | 47.3 | 47.1 | 1.8 | 9.8 | 5.1 |
| Progression Factor | 1.00 | 1.00 | 0.99 | 0.93 | 0.67 | 0.85 |
| Incremental Delay, d2 | 0.4 | 2.1 | 4.0 | 0.2 | 1.4 | 0.0 |
| Delay (s) | 46.4 | 49.3 | 50.7 | 1.9 | 7.9 | 4.3 |
| Level of Service | D | D | D | A | A | A |
| Approach Delay (s) | 49.1 | | | 8.6 | 7.9 | |
| Approach LOS | D | | | A | A | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | | | 12.1 | | HCM Level of Service | B |
| HCM Volume to Capacity ratio | | | 0.78 | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | | | 64.0% | | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
|-----------------------------------|------|------|------|-------|------|------|----------------------|------|------|------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ | ↕ | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Frt | 1.00 | 0.85 | 0.93 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | |
| Flt Protected | 0.96 | 1.00 | 0.98 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | |
| Satd. Flow (prot) | 1782 | 1583 | 1698 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | |
| Flt Permitted | 0.74 | 1.00 | 0.85 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | |
| Satd. Flow (perm) | 1378 | 1583 | 1473 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | |
| Volume (vph) | 26 | 3 | 137 | 23 | 3 | 27 | 94 | 587 | 25 | 10 | 1825 | 20 | |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | |
| Adj. Flow (vph) | 28 | 3 | 149 | 25 | 3 | 29 | 102 | 638 | 27 | 11 | 1984 | 22 | |
| RTOR Reduction (vph) | 0 | 0 | 108 | 0 | 27 | 0 | 0 | 0 | 5 | 0 | 0 | 6 | |
| Lane Group Flow (vph) | 0 | 31 | 41 | 0 | 30 | 0 | 102 | 638 | 22 | 11 | 1984 | 16 | |
| Turn Type | Perm | Perm | Perm | Perm | Perm | Prot | Perm | Prot | Perm | Prot | Perm | Perm | |
| Protected Phases | | 4 | | 8 | | 5 | 2 | | 2 | | 1 | 6 | |
| Permitted Phases | 4 | | 4 | 8 | | | | | | | | 6 | |
| Actuated Green, G (s) | | 8.9 | 8.9 | | 8.9 | | 10.3 | 87.9 | 87.9 | | 1.2 | 78.8 | 78.8 |
| Effective Green, g (s) | | 8.9 | 8.9 | | 8.9 | | 10.3 | 87.9 | 87.9 | | 1.2 | 78.8 | 78.8 |
| Actuated g/C Ratio | | 0.08 | 0.08 | | 0.08 | | 0.09 | 0.80 | 0.80 | | 0.01 | 0.72 | 0.72 |
| Clearance Time (s) | | 4.0 | 4.0 | | 4.0 | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | | 3.0 | 3.0 | | 3.0 | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 111 | 128 | | 119 | | 166 | 2828 | 1265 | | 19 | 2535 | 1134 |
| v/s Ratio Prot | | | | | | | c0.06 | 0.18 | | | 0.01 | c0.56 | |
| v/s Ratio Perm | | 0.02 | 0.09 | | 0.04 | | | | 0.02 | | | | 0.01 |
| v/c Ratio | | 0.28 | 0.32 | | 0.26 | | 0.61 | 0.23 | 0.02 | | 0.58 | 0.78 | 0.01 |
| Uniform Delay, d1 | | 47.5 | 47.7 | | 47.4 | | 47.9 | 2.7 | 2.3 | | 54.1 | 10.1 | 4.5 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 1.00 | 1.00 | 1.00 | | 1.30 | 0.23 | 0.05 |
| Incremental Delay, d2 | | 1.4 | 1.5 | | 1.1 | | 6.6 | 0.2 | 0.0 | | 28.2 | 1.9 | 0.0 |
| Delay (s) | | 48.9 | 49.2 | | 48.6 | | 54.5 | 2.9 | 2.3 | | 98.6 | 4.2 | 0.2 |
| Level of Service | | D | D | | D | | D | A | A | | F | A | A |
| Approach Delay (s) | | 49.1 | | | 48.6 | | | 9.7 | | | | 4.7 | |
| Approach LOS | | D | | | D | | | A | | | | A | |
| Intersection Summary | | | | | | | | | | | | | |
| HCM Average Control Delay | | | | 9.4 | | | HCM Level of Service | | | | A | | |
| HCM Volume to Capacity ratio | | | | 0.80 | | | | | | | | | |
| Actuated Cycle Length (s) | | | | 110.0 | | | Sum of lost time (s) | | | | 12.0 | | |
| Intersection Capacity Utilization | | | | 75.4% | | | ICU Level of Service | | | | D | | |
| Analysis Period (min) | | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-------|-------|-------|------|-------|------|------|------|------|-------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.96 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1787 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1787 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 92 | 54 | 20 | 170 | 72 | 32 | 108 | 387 | 125 | 148 | 1380 | 728 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 100 | 59 | 22 | 185 | 78 | 35 | 117 | 421 | 136 | 161 | 1500 | 791 |
| RTOR Reduction (vph) | 0 | 14 | 0 | 0 | 0 | 31 | 0 | 0 | 68 | 0 | 0 | 308 |
| Lane Group Flow (vph) | 100 | 67 | 0 | 128 | 135 | 4 | 117 | 421 | 68 | 161 | 1500 | 483 |
| Turn Type | Split | | Split | | Perm | | Prot | | Prot | | Perm | |
| Protected Phases | 4 | 4 | 8 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | 8 | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 9.4 | 9.4 | 13.5 | | 13.5 | 9.6 | | 53.5 | 53.5 | 14.6 | 58.5 | 58.5 |
| Effective Green, g (s) | 9.4 | 9.4 | 13.5 | | 13.5 | 9.6 | | 53.5 | 53.5 | 14.6 | 58.5 | 58.5 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.13 | | 0.13 | 0.09 | | 0.50 | 0.50 | 0.14 | 0.55 | 0.55 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 302 | 157 | 212 | | 219 | 200 | | 159 | 1770 | 792 | 1935 | 865 |
| v/s Ratio Prot | 0.03 | c0.05 | 0.08 | | c0.08 | 0.07 | | 0.12 | | c0.09 | 0.42 | |
| v/s Ratio Perm | | | | | | 0.02 | | | 0.09 | | | 0.50 |
| v/c Ratio | 0.33 | 0.43 | 0.60 | | 0.62 | 0.02 | | 0.74 | 0.24 | 0.09 | 0.67 | 0.78 |
| Uniform Delay, d1 | 45.8 | 46.3 | 44.2 | | 44.3 | 41.0 | | 47.5 | 15.2 | 14.0 | 43.9 | 19.1 |
| Progression Factor | 1.00 | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.6 | 1.9 | 4.8 | | 5.1 | 0.0 | | 16.2 | 0.3 | 0.2 | 6.7 | 3.1 |
| Delay (s) | 46.5 | 48.1 | 49.0 | | 49.4 | 41.0 | | 63.6 | 15.5 | 14.2 | 50.6 | 22.2 |
| Level of Service | D | D | D | | D | D | | E | B | B | D | C |
| Approach Delay (s) | 47.2 | | 48.2 | | 23.6 | | 22.8 | | | | | |
| Approach LOS | D | | D | | C | | C | | | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|----------------------|------|
| HCM Average Control Delay | 26.3 | HCM Level of Service | C |
| HCM Volume to Capacity ratio | 0.79 | | |
| Actuated Cycle Length (s) | 107.0 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 67.7% | ICU Level of Service | C |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|------------------------|-------|------|------|------|-------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 212 | 216 | 60 | 0 | 0 | 0 | 224 | 688 | 396 | 0 | 492 | 528 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 230 | 235 | 65 | 0 | 0 | 0 | 243 | 748 | 430 | 0 | 535 | 574 |
| RTOR Reduction (vph) | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 80 | 0 | 0 | 312 |
| Lane Group Flow (vph) | 230 | 274 | 0 | 0 | 0 | 0 | 243 | 748 | 350 | 0 | 535 | 262 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | | | 5 | | 2 | | | 6 | | |
| Permitted Phases | | | | | 2 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 18.8 | 18.8 | | | 29.0 | | 83.2 | 83.2 | 50.2 | | 50.2 | |
| Effective Green, g (s) | 18.8 | 18.8 | | | 29.0 | | 83.2 | 83.2 | 50.2 | | 50.2 | |
| Actuated g/C Ratio | 0.17 | 0.17 | | | 0.26 | | 0.76 | 0.76 | 0.46 | | 0.46 | |
| Clearance Time (s) | 4.0 | 4.0 | | | 4.0 | | 4.0 | 4.0 | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | 3.0 | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 303 | 585 | | | 467 | | 2677 | 1197 | 1615 | | 722 | |
| v/s Ratio Prot | c0.13 | 0.09 | | | c0.14 | | 0.21 | | 0.15 | | | |
| v/s Ratio Perm | | | | | | | | 0.27 | | | 0.36 | |
| v/c Ratio | 0.76 | 0.47 | | | 0.52 | | 0.28 | 0.29 | 0.33 | | 0.36 | |
| Uniform Delay, d1 | 43.4 | 41.1 | | | 34.6 | | 4.1 | 4.2 | 19.1 | | 19.5 | |
| Progression Factor | 1.00 | 1.00 | | | 0.99 | | 0.77 | 0.40 | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 10.4 | 0.6 | | | 1.0 | | 0.3 | 0.6 | 0.6 | | 1.4 | |
| Delay (s) | 53.9 | 41.7 | | | 35.3 | | 3.4 | 2.3 | 19.7 | | 20.9 | |
| Level of Service | D | D | | | D | | A | A | B | | C | |
| Approach Delay (s) | 47.0 | | | | 0.0 | | 8.5 | | 20.3 | | | |
| Approach LOS | D | | | | A | | A | | C | | | |

| Intersection Summary | | | |
|-----------------------------------|-------|----------------------|------|
| HCM Average Control Delay | 19.5 | HCM Level of Service | B |
| HCM Volume to Capacity ratio | 0.71 | | |
| Actuated Cycle Length (s) | 110.0 | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | 51.8% | ICU Level of Service | A |
| Analysis Period (min) | 15 | | |
| c Critical Lane Group | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1833 | 1583 | 1770 | 1583 | 1583 | 1583 | 3529 | 3529 | 1770 | 1770 | 3539 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.72 | 1.00 | 1.00 | 1.00 | 1.00 | 0.24 | 1.00 | 0.24 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1833 | 1583 | 1347 | 1583 | 1583 | 1583 | 3529 | 3529 | 446 | 446 | 3539 | 3539 |
| Volume (vph) | 16 | 32 | 132 | 12 | 0 | 136 | 0 | 1020 | 20 | 80 | 448 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 17 | 35 | 143 | 13 | 0 | 148 | 0 | 1109 | 22 | 87 | 487 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 131 | 0 | 0 | 96 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 52 | 12 | 13 | 0 | 52 | 0 | 1131 | 0 | 87 | 487 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | | | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 9.1 | | | 9.1 | | | 9.1 | | | 92.9 | | |
| Effective Green, g (s) | 9.1 | | | 9.1 | | | 9.1 | | | 92.9 | | |
| Actuated g/C Ratio | 0.08 | | | 0.08 | | | 0.08 | | | 0.84 | | |
| Clearance Time (s) | 4.0 | | | 4.0 | | | 4.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | | | 3.0 | | | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 152 | | | 131 | | | 111 | | | 2980 | | |
| v/s Ratio Prot | 0.03 | | | 0.09 | | | 0.01 | | | 0.09 | | |
| v/c Ratio | 0.34 | | | 0.09 | | | 0.12 | | | 0.39 | | |
| Uniform Delay, d1 | 47.6 | | | 46.6 | | | 46.7 | | | 47.8 | | |
| Progression Factor | 1.00 | | | 1.00 | | | 1.00 | | | 1.00 | | |
| Incremental Delay, d2 | 1.3 | | | 0.3 | | | 0.5 | | | 2.0 | | |
| Delay (s) | 49.0 | | | 46.9 | | | 47.2 | | | 49.8 | | |
| Level of Service | D | | | D | | | D | | | A | | |
| Approach Delay (s) | 47.5 | | | 49.6 | | | 2.3 | | | 1.3 | | |
| Approach LOS | D | | | D | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 10.0 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.45 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 52.5% | | | ICU Level of Service | | | A | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Existing AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 26 | 200 | 48 | 172 | 645 | 36 | 85 | 12 | 76 | 28 | 8 | 52 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 28 | 217 | 52 | 187 | 701 | 39 | 92 | 13 | 83 | 30 | 9 | 57 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | None | | | | | | None | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 740 | | | 270 | | | 1436 | | | 1414 | | |
| vC1, stage 1 conf vol | | | | | | | 243 | | | 1375 | | |
| vC2, stage 2 conf vol | | | | | | | 1436 | | | 1414 | | |
| vCu, unblocked vol | 740 | | | 270 | | | 1436 | | | 1414 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | |
| tC, 2 stage (s) | | | | | | | 3.5 | | | 4.0 | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | |
| p0 queue free % | 97 | | | 86 | | | 0 | | | 89 | | |
| cM capacity (veh/h) | 866 | | | 1294 | | | 79 | | | 114 | | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 28 | 270 | 187 | 740 | 105 | 83 | 96 | | | | | |
| Volume Left | 28 | 0 | 187 | 0 | 92 | 0 | 30 | | | | | |
| Volume Right | 0 | 52 | 0 | 39 | 0 | 83 | 57 | | | | | |
| cSH | 866 | 1700 | 1294 | 1700 | 82 | 795 | 171 | | | | | |
| Volume to Capacity | 0.03 | 0.16 | 0.14 | 0.44 | 1.28 | 0.10 | 0.56 | | | | | |
| Queue Length (ft) | 3 | 0 | 13 | 0 | 198 | 9 | 73 | | | | | |
| Control Delay (s) | 9.3 | 0.0 | 8.3 | 0.0 | 284.5 | 10.1 | 49.7 | | | | | |
| Lane LOS | A | A | A | F | B | E | E | | | | | |
| Approach Delay (s) | 0.9 | | 1.7 | | 163.9 | | 49.7 | | | | | |
| Approach LOS | F | | E | | F | | E | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 24.8 | | | | | | | | | | | |
| Intersection Capacity Utilization | 61.5% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|-------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | | ↔ | | | ↔ | | | ↔ | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | |
| Frt | 0.90 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1653 | | 1770 | | 3539 | | 3527 | | 3527 | | 3527 | |
| Flt Permitted | 0.96 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1602 | | 1770 | | 3539 | | 3527 | | 3527 | | 3527 | |
| Volume (vph) | 6 | 0 | 19 | 0 | 0 | 0 | 18 | 1898 | 0 | 0 | 724 | 17 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 7 | 0 | 21 | 0 | 0 | 0 | 20 | 2063 | 0 | 0 | 787 | 18 |
| RTOR Reduction (vph) | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 10 | 0 | 0 | 0 | 0 | 20 | 2063 | 0 | 0 | 804 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | | |
| Permitted Phases | | | | | | | | | | | | |
| Actuated Green, G (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Effective Green, g (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Actuated g/C Ratio | 0.16 | | 0.03 | | 0.76 | | 0.70 | | 0.70 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 262 | | 45 | | 2703 | | 2475 | | 2475 | | 2475 | |
| v/s Ratio Prot | | | 0.01 | | c0.58 | | 0.23 | | 0.23 | | 0.23 | |
| v/s Ratio Perm | c0.02 | | | | | | | | | | | |
| v/c Ratio | 0.04 | | 0.44 | | 0.76 | | 0.32 | | 0.32 | | 0.32 | |
| Uniform Delay, d1 | 38.7 | | 52.8 | | 7.4 | | 6.3 | | 6.3 | | 6.3 | |
| Progression Factor | 1.00 | | 0.96 | | 0.86 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.1 | | 4.7 | | 1.4 | | 0.3 | | 0.3 | | 0.3 | |
| Delay (s) | 38.8 | | 55.5 | | 7.8 | | 6.7 | | 6.7 | | 6.7 | |
| Level of Service | D | | E | | A | | A | | A | | A | |
| Approach Delay (s) | 38.8 | | 0.0 | | 8.3 | | 6.7 | | 6.7 | | 6.7 | |
| Approach LOS | D | | A | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.1 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.65 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | B | | B | | B | | B | |
| Intersection Capacity Utilization | 62.5% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|-------|------|------|------|
| Lane Configurations | ↔ | | | ↔ | | | ↔ | | | ↔ | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Frt | 0.96 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.96 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (prot) | 1731 | | 1770 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Flt Permitted | 0.88 | | 0.75 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (perm) | 1580 | | 1398 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Volume (vph) | 7 | 0 | 3 | 12 | 0 | 16 | 3 | 1854 | 21 | 11 | 706 | 4 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 8 | 0 | 3 | 13 | 0 | 17 | 3 | 2015 | 23 | 12 | 767 | 4 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 14 | 0 | 0 | 5 | 0 | 0 | 1 |
| Lane Group Flow (vph) | 0 | 8 | 0 | 0 | 13 | 3 | 3 | 2015 | 18 | 12 | 767 | 3 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.01 | | 0.73 | | 0.73 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 241 | | 214 | | 242 | | 19 | | 2574 | | 1151 | |
| v/s Ratio Prot | | | 0.01 | | 0.01 | | 0.00 | | c0.57 | | 0.22 | |
| v/s Ratio Perm | 0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.00 | |
| v/c Ratio | 0.04 | | 0.06 | | 0.01 | | 0.16 | | 0.78 | | 0.02 | |
| Uniform Delay, d1 | 39.7 | | 39.9 | | 39.5 | | 53.9 | | 9.5 | | 4.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.09 | | 0.82 | | 0.89 | |
| Incremental Delay, d2 | 0.1 | | 0.1 | | 0.0 | | 2.9 | | 1.8 | | 0.0 | |
| Delay (s) | 39.8 | | 40.0 | | 39.6 | | 61.4 | | 9.6 | | 3.7 | |
| Level of Service | D | | D | | E | | A | | A | | F | |
| Approach Delay (s) | 39.8 | | 39.7 | | 9.6 | | 5.4 | | 5.4 | | 5.4 | |
| Approach LOS | D | | D | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.9 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.66 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | | C | | C | | C | |
| Intersection Capacity Utilization | 67.9% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|-------|----------------------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 20 | 103 | 182 | 1864 | 718 | 26 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 22 | 112 | 198 | 2026 | 780 | 28 |
| RTOR Reduction (vph) | 0 | 105 | 0 | 0 | 0 | 10 |
| Lane Group Flow (vph) | 22 | 7 | 198 | 2026 | 780 | 18 |
| Turn Type | Perm | Prot | Prot | Perm | Perm | Perm |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | | 4 | | | | 6 |
| Actuated Green, G (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Effective Green, g (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Actuated g/C Ratio | 0.07 | 0.07 | 0.18 | 0.86 | 0.65 | 0.65 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 117 | 105 | 317 | 3047 | 2284 | 1022 |
| v/s Ratio Prot | 0.01 | | 0.11 | c0.57 | 0.22 | |
| v/s Ratio Perm | | 0.07 | | | 0.02 | |
| v/c Ratio | 0.19 | 0.07 | 0.62 | 0.66 | 0.34 | 0.02 |
| Uniform Delay, d1 | 48.5 | 48.2 | 41.7 | 2.5 | 8.9 | 7.0 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.33 | 2.64 |
| Incremental Delay, d2 | 0.8 | 0.3 | 3.8 | 1.2 | 0.4 | 0.0 |
| Delay (s) | 49.3 | 48.5 | 45.5 | 3.7 | 12.2 | 18.5 |
| Level of Service | D | D | D | A | B | B |
| Approach Delay (s) | 48.6 | | | 7.4 | 12.4 | |
| Approach LOS | D | | | A | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | | | 10.4 | | HCM Level of Service | B |
| HCM Volume to Capacity ratio | | | 0.69 | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 8.0 |
| Intersection Capacity Utilization | | | 61.5% | | ICU Level of Service | B |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|------|------|-------|------|------|-------|-------|----------------------|-------|-------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | |
| Flt Protected | 0.96 | 1.00 | | 0.96 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1781 | 1583 | | 1739 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Flt Permitted | 0.80 | 1.00 | | 0.76 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1483 | 1583 | | 1371 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Volume (vph) | 19 | 2 | 87 | 22 | 0 | 6 | 123 | 2017 | 33 | 14 | 846 | 24 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 21 | 2 | 95 | 24 | 0 | 7 | 134 | 2192 | 36 | 15 | 920 | 26 |
| RTOR Reduction (vph) | 0 | 0 | 90 | 0 | 7 | 0 | 0 | 0 | 5 | 0 | 0 | 7 |
| Lane Group Flow (vph) | 0 | 23 | 5 | 0 | 24 | 0 | 134 | 2192 | 31 | 15 | 920 | 19 |
| Turn Type | Perm | Perm | Perm | Perm | Prot | Perm | Prot | Perm | Prot | Prot | Perm | Perm |
| Protected Phases | | 4 | | | 8 | | 5 | 2 | | 2 | 1 | 6 |
| Permitted Phases | 4 | | 4 | 8 | | | | | 2 | | | 6 |
| Actuated Green, G (s) | | 8.1 | 8.1 | | 8.1 | | 25.4 | 126.7 | 126.7 | 3.2 | 104.5 | 104.5 |
| Effective Green, g (s) | | 8.1 | 8.1 | | 8.1 | | 25.4 | 126.7 | 126.7 | 3.2 | 104.5 | 104.5 |
| Actuated g/C Ratio | | 0.05 | 0.05 | | 0.05 | | 0.17 | 0.84 | 0.84 | 0.02 | 0.70 | 0.70 |
| Clearance Time (s) | | 4.0 | 4.0 | | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | | 3.0 | 3.0 | | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | | 80 | 85 | | 74 | | 300 | 2989 | 1337 | 38 | 2466 | 1103 |
| v/s Ratio Prot | | | | | | | 0.08 | c0.62 | | 0.01 | c0.26 | |
| v/s Ratio Perm | | 0.02 | 0.06 | | 0.02 | | | | 0.02 | | | 0.02 |
| v/c Ratio | | 0.29 | 0.06 | | 0.33 | | 0.45 | 0.73 | 0.02 | 0.39 | 0.37 | 0.02 |
| Uniform Delay, d1 | | 68.2 | 67.3 | | 68.3 | | 56.0 | 4.8 | 1.8 | 72.4 | 9.3 | 7.0 |
| Progression Factor | | 1.00 | 1.00 | | 1.00 | | 0.78 | 0.06 | 0.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | | 2.0 | 0.3 | | 2.6 | | 0.4 | 0.7 | 0.0 | 6.6 | 0.4 | 0.0 |
| Delay (s) | | 70.2 | 67.6 | | 70.9 | | 44.0 | 1.0 | 0.0 | 79.1 | 9.8 | 7.0 |
| Level of Service | | E | E | | E | | D | A | A | E | A | A |
| Approach Delay (s) | | 68.1 | | | 70.9 | | | 3.4 | | | 10.8 | |
| Approach LOS | | E | | | E | | | A | | | B | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | | | | 8.2 | | | | | HCM Level of Service | A | |
| HCM Volume to Capacity ratio | | | | | 0.75 | | | | | | | |
| Actuated Cycle Length (s) | | | | | 150.0 | | | | | Sum of lost time (s) | 12.0 | |
| Intersection Capacity Utilization | | | | | 77.3% | | | | | ICU Level of Service | D | |
| Analysis Period (min) | | | | | 15 | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|------|------|------|-------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.90 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 502 | 64 | 112 | 103 | 61 | 112 | 365 | 1697 | 72 | 86 | 725 | 94 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 546 | 70 | 122 | 112 | 66 | 122 | 397 | 1845 | 78 | 93 | 788 | 102 |
| RTOR Reduction (vph) | 0 | 42 | 0 | 0 | 0 | 97 | 0 | 0 | 21 | 0 | 0 | 62 |
| Lane Group Flow (vph) | 546 | 150 | 0 | 87 | 91 | 25 | 397 | 1845 | 57 | 93 | 788 | 40 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | | | 8 | | | 2 | | | | 6 |
| Actuated Green, G (s) | 22.0 | 22.0 | | 12.6 | 12.6 | 12.6 | 47.0 | 88.4 | 88.4 | 11.0 | 52.4 | 52.4 |
| Effective Green, g (s) | 22.0 | 22.0 | | 12.6 | 12.6 | 12.6 | 47.0 | 88.4 | 88.4 | 11.0 | 52.4 | 52.4 |
| Actuated g/C Ratio | 0.15 | 0.15 | | 0.08 | 0.08 | 0.08 | 0.31 | 0.59 | 0.59 | 0.07 | 0.35 | 0.35 |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 504 | 247 | | 141 | 147 | 133 | 555 | 2086 | 933 | 130 | 1236 | 553 |
| v/s Ratio Prot | c0.16 | 0.11 | | 0.05 | 0.05 | | 0.22 | c0.52 | | 0.05 | c0.22 | |
| v/s Ratio Perm | | | | | 0.08 | | | 0.05 | | | | 0.06 |
| v/c Ratio | 1.08 | 0.61 | | 0.62 | 0.62 | 0.19 | 0.72 | 0.88 | 0.06 | 0.72 | 0.64 | 0.07 |
| Uniform Delay, d1 | 64.0 | 60.0 | | 66.4 | 66.4 | 63.9 | 45.6 | 26.4 | 13.1 | 68.0 | 40.9 | 32.6 |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 0.90 | 0.83 | 1.07 | 0.88 | 0.83 | 1.54 |
| Incremental Delay, d2 | 64.5 | 4.2 | | 7.8 | 7.5 | 0.7 | 4.2 | 5.8 | 0.1 | 16.3 | 2.4 | 0.2 |
| Delay (s) | 128.5 | 64.2 | | 74.2 | 73.9 | 64.6 | 45.2 | 27.7 | 14.1 | 76.3 | 36.4 | 50.5 |
| Level of Service | F | E | | E | E | E | D | C | B | E | D | D |
| Approach Delay (s) | 111.8 | | | 70.2 | | | 30.2 | | | 41.7 | | |
| Approach LOS | F | | | E | | | C | | | D | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 49.4 | | | HCM Level of Service | | | D | | | | | |
| HCM Volume to Capacity ratio | 0.91 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 16.0 | | | | | |
| Intersection Capacity Utilization | 83.8% | | | ICU Level of Service | | | E | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|------|------|-------|-------|-------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.93 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 389 | 241 | 192 | 0 | 0 | 0 | 207 | 915 | 411 | 0 | 605 | 341 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 423 | 262 | 209 | 0 | 0 | 0 | 225 | 995 | 447 | 0 | 658 | 371 |
| RTOR Reduction (vph) | 0 | 116 | 0 | 0 | 0 | 0 | 0 | 0 | 140 | 0 | 0 | 217 |
| Lane Group Flow (vph) | 423 | 355 | 0 | 0 | 0 | 0 | 225 | 995 | 307 | 0 | 658 | 154 |
| Turn Type | Prot | | | Prot | | Perm | Prot | | Perm | | Prot | Perm |
| Protected Phases | 7 | 4 | | | | | 5 | 2 | | | 6 | |
| Permitted Phases | | | | | | | | 2 | | | | 6 |
| Actuated Green, G (s) | 38.9 | 38.9 | | | | | 37.0 | 103.1 | 103.1 | | 62.1 | 62.1 |
| Effective Green, g (s) | 38.9 | 38.9 | | | | | 37.0 | 103.1 | 103.1 | | 62.1 | 62.1 |
| Actuated g/C Ratio | 0.26 | 0.26 | | | | | 0.25 | 0.69 | 0.69 | | 0.41 | 0.41 |
| Clearance Time (s) | 4.0 | 4.0 | | | | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | | | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 459 | 857 | | | | | 437 | 2432 | 1088 | | 1465 | 655 |
| v/s Ratio Prot | c0.24 | 0.14 | | | | | c0.13 | 0.28 | | | 0.19 | |
| v/s Ratio Perm | | | | | | | | 0.28 | | | | 0.23 |
| v/c Ratio | 0.92 | 0.41 | | | | | 0.51 | 0.41 | 0.28 | | 0.45 | 0.23 |
| Uniform Delay, d1 | 54.1 | 46.1 | | | | | 48.8 | 10.2 | 9.1 | | 31.6 | 28.5 |
| Progression Factor | 1.00 | 1.00 | | | | | 0.97 | 0.83 | 0.35 | | 0.23 | 0.02 |
| Incremental Delay, d2 | 23.9 | 0.3 | | | | | 1.0 | 0.5 | 0.6 | | 0.8 | 0.7 |
| Delay (s) | 78.0 | 46.4 | | | | | 48.0 | 8.9 | 3.8 | | 8.1 | 1.2 |
| Level of Service | E | D | | | | | D | A | A | | A | A |
| Approach Delay (s) | 61.4 | | | 0.0 | | | 12.8 | | | 5.6 | | |
| Approach LOS | E | | | A | | | B | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 22.8 | | | HCM Level of Service | | | C | | | | | |
| HCM Volume to Capacity ratio | 0.65 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 59.7% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Satd. Flow (prot) | 1831 | 1583 | 1770 | 1583 | 1583 | 3528 | 3528 | 1770 | 3539 | 1770 | 3539 | 1770 |
| Flt Permitted | 0.98 | 1.00 | 0.46 | 1.00 | 1.00 | 1.00 | 1.00 | 0.17 | 1.00 | 0.17 | 1.00 | 0.17 |
| Satd. Flow (perm) | 1831 | 1583 | 849 | 1583 | 1583 | 3528 | 3528 | 322 | 3539 | 322 | 3539 | 322 |
| Volume (vph) | 35 | 64 | 175 | 17 | 0 | 31 | 0 | 1272 | 27 | 121 | 570 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 38 | 70 | 190 | 18 | 0 | 34 | 0 | 1383 | 29 | 132 | 620 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 172 | 0 | 0 | 31 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 108 | 18 | 18 | 0 | 3 | 0 | 1411 | 0 | 132 | 620 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | 4 | 8 | 8 | 8 | 6 | 6 | 6 | 6 | 6 | 6 | 6 |
| Actuated Green, G (s) | 13.9 | 13.9 | 13.9 | 13.9 | 13.9 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 |
| Effective Green, g (s) | 13.9 | 13.9 | 13.9 | 13.9 | 13.9 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 170 | 147 | 79 | 147 | 147 | 3013 | 275 | 3022 | 275 | 3022 | 275 | 3022 |
| v/s Ratio Prot | | | | | | 0.40 | 0.40 | 0.18 | 0.40 | 0.18 | 0.18 | 0.18 |
| v/s Ratio Perm | 0.06 | 0.12 | 0.02 | 0.02 | 0.02 | c0.41 | c0.41 | 0.21 | c0.41 | 0.21 | 0.21 | 0.21 |
| v/c Ratio | 0.64 | 0.12 | 0.23 | 0.02 | 0.02 | 0.47 | 0.48 | 0.21 | 0.47 | 0.21 | 0.21 | 0.21 |
| Uniform Delay, d1 | 65.6 | 62.4 | 63.1 | 61.9 | 61.9 | 2.7 | 2.7 | 1.9 | 2.7 | 1.9 | 1.9 | 1.9 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.44 | 0.29 | 1.00 | 0.29 | 0.29 | 0.29 |
| Incremental Delay, d2 | 7.5 | 0.4 | 1.5 | 0.1 | 0.1 | 0.5 | 5.4 | 0.1 | 0.5 | 0.1 | 0.1 | 0.1 |
| Delay (s) | 73.2 | 62.8 | 64.5 | 61.9 | 61.9 | 3.2 | 9.4 | 0.7 | 3.2 | 0.7 | 0.7 | 0.7 |
| Level of Service | E | E | E | E | E | A | A | A | A | A | A | A |
| Approach Delay (s) | 66.6 | | | 62.8 | | 3.2 | | 2.2 | 3.2 | | 2.2 | |
| Approach LOS | E | | | E | | A | | A | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 11.6 | | | HCM Level of Service | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.56 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 64.7% | | | ICU Level of Service | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Existing PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|-------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 46 | 612 | 49 | 104 | 292 | 85 | 57 | 8 | 107 | 61 | 12 | 19 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 50 | 665 | 53 | 113 | 317 | 92 | 62 | 9 | 116 | 66 | 13 | 21 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | None | | | | | | None | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 410 | 718 | | | 1362 | | | 1428 | 692 | 1359 | 1408 | 364 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 410 | 718 | | | 1362 | | | 1428 | 692 | 1359 | 1408 | 364 |
| tC, single (s) | 4.1 | 4.1 | | | 7.1 | | | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | 2.2 | | | 3.5 | | | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 96 | 87 | | | 36 | | | 92 | 74 | 13 | 89 | 97 |
| cM capacity (veh/h) | 1149 | 883 | | | 97 | | | 113 | 444 | 76 | 116 | 681 |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 50 | 718 | 113 | 410 | 71 | 116 | 100 | | | | | |
| Volume Left | 50 | 0 | 113 | 0 | 62 | 0 | 66 | | | | | |
| Volume Right | 0 | 53 | 0 | 92 | 0 | 116 | 21 | | | | | |
| cSH | 1149 | 1700 | 883 | 1700 | 99 | 444 | 99 | | | | | |
| Volume to Capacity | 0.04 | 0.42 | 0.13 | 0.24 | 0.72 | 0.26 | 1.01 | | | | | |
| Queue Length (ft) | 3 | 0 | 11 | 0 | 92 | 26 | 155 | | | | | |
| Control Delay (s) | 8.3 | 0.0 | 9.7 | 0.0 | 103.5 | 16.0 | 172.9 | | | | | |
| Lane LOS | A | A | A | F | C | F | F | | | | | |
| Approach Delay (s) | 0.5 | 2.1 | | 49.1 | | 172.9 | | | | | | |
| Approach LOS | E | E | | F | | F | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 17.7 | | | | | | | | | | | |
| Intersection Capacity Utilization | 62.8% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|-------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 1.00 | | 1.00 | |
| Frt | 0.86 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Flt Permitted | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Volume (vph) | 0 | 0 | 34 | 0 | 0 | 0 | 15 | 547 | 0 | 0 | 1398 | 9 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 37 | 0 | 0 | 0 | 16 | 595 | 0 | 0 | 1520 | 10 |
| RTOR Reduction (vph) | 0 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 1 | 0 | 0 | 0 | 0 | 16 | 595 | 0 | 0 | 1530 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Effective Green, g (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Actuated g/C Ratio | 0.03 | | 0.03 | | 0.90 | | 0.83 | | 0.83 | | 0.83 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 48 | | 50 | | 3175 | | 2945 | | 2945 | | 2945 | |
| v/s Ratio Prot | c0.02 | | c0.01 | | 0.17 | | c0.43 | | c0.43 | | c0.43 | |
| v/s Ratio Perm | 0.02 | | 0.32 | | 0.19 | | 0.52 | | 0.52 | | 0.52 | |
| Uniform Delay, d1 | 51.8 | | 52.4 | | 0.7 | | 2.7 | | 2.7 | | 2.7 | |
| Progression Factor | 1.00 | | 0.81 | | 0.71 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.2 | | 3.6 | | 0.1 | | 0.7 | | 0.7 | | 0.7 | |
| Delay (s) | 52.0 | | 46.1 | | 0.6 | | 3.4 | | 3.4 | | 3.4 | |
| Level of Service | D | | D | | A | | A | | A | | A | |
| Approach Delay (s) | 52.0 | | 0.0 | | 1.8 | | 3.4 | | 3.4 | | 3.4 | |
| Approach LOS | D | | A | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 3.8 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.52 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | A | | A | | A | | A | |
| Intersection Capacity Utilization | 48.9% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | |
| Frt | 0.90 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1659 | | 1770 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Flt Permitted | 0.96 | | 0.75 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (perm) | 1618 | | 1403 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Volume (vph) | 2 | 0 | 5 | 17 | 0 | 9 | 0 | 615 | 12 | 7 | 1555 | 1 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 2 | 0 | 5 | 18 | 0 | 10 | 0 | 668 | 13 | 8 | 1690 | 1 |
| RTOR Reduction (vph) | 0 | 4 | 0 | 0 | 0 | 8 | 0 | 0 | 4 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 3 | 0 | 0 | 18 | 2 | 0 | 668 | 9 | 8 | 1690 | 1 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.73 | | 0.73 | | 0.01 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 247 | | 214 | | 242 | | 2574 | | 1151 | | 19 | |
| v/s Ratio Prot | 0.00 | | c0.01 | | 0.01 | | 0.19 | | 0.01 | | c0.48 | |
| v/s Ratio Perm | 0.01 | | 0.08 | | 0.01 | | 0.26 | | 0.01 | | 0.62 | |
| Uniform Delay, d1 | 39.6 | | 40.0 | | 39.5 | | 5.0 | | 4.1 | | 54.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.25 | | 1.85 | | 1.23 | |
| Incremental Delay, d2 | 0.0 | | 0.2 | | 0.0 | | 0.2 | | 0.0 | | 13.2 | |
| Delay (s) | 39.6 | | 40.2 | | 39.5 | | 6.5 | | 7.6 | | 79.8 | |
| Level of Service | D | | D | | D | | A | | A | | E | |
| Approach Delay (s) | 39.6 | | 39.9 | | 6.6 | | 5.0 | | 5.0 | | 5.0 | |
| Approach LOS | D | | D | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 5.9 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.53 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | A | | A | | A | | A | |
| Intersection Capacity Utilization | 53.0% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|------|----------------------|------|
| Lane Configurations | | | | | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 22 | 224 | 94 | 616 | 1653 | 10 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 24 | 243 | 102 | 670 | 1797 | 11 |
| RTOR Reduction (vph) | 0 | 177 | 0 | 0 | 0 | 3 |
| Lane Group Flow (vph) | 24 | 66 | 102 | 670 | 1797 | 8 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | 4 | | | | 6 | |
| Actuated Green, G (s) | 10.2 | 10.2 | 11.2 | 91.8 | 76.6 | 76.6 |
| Effective Green, g (s) | 10.2 | 10.2 | 11.2 | 91.8 | 76.6 | 76.6 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.10 | 0.83 | 0.70 | 0.70 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 164 | 147 | 180 | 2953 | 2464 | 1102 |
| v/s Ratio Prot | 0.01 | | c0.06 | 0.19 | c0.51 | |
| v/s Ratio Perm | | 0.15 | | | | 0.01 |
| v/c Ratio | 0.15 | 0.45 | 0.57 | 0.23 | 0.73 | 0.01 |
| Uniform Delay, d1 | 45.9 | 47.2 | 47.1 | 1.9 | 10.3 | 5.1 |
| Progression Factor | 1.00 | 1.00 | 0.99 | 0.93 | 0.69 | 0.91 |
| Incremental Delay, d2 | 0.4 | 2.2 | 4.0 | 0.2 | 1.6 | 0.0 |
| Delay (s) | 46.3 | 49.4 | 50.5 | 1.9 | 8.7 | 4.6 |
| Level of Service | D | D | D | A | A | A |
| Approach Delay (s) | 49.1 | | | 8.3 | 8.7 | |
| Approach LOS | D | | | A | A | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | | | 12.4 | | HCM Level of Service | B |
| HCM Volume to Capacity ratio | | | 0.81 | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | | | 66.2% | | ICU Level of Service | C |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|-------|------|----------------------|------|------|------|-------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.93 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.98 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1782 | 1583 | 1698 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.74 | 1.00 | 0.85 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1378 | 1583 | 1473 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 26 | 3 | 137 | 23 | 3 | 27 | 94 | 616 | 25 | 10 | 1916 | 20 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 28 | 3 | 149 | 25 | 3 | 29 | 102 | 670 | 27 | 11 | 2083 | 22 |
| RTOR Reduction (vph) | 0 | 0 | 108 | 0 | 27 | 0 | 0 | 0 | 5 | 0 | 0 | 6 |
| Lane Group Flow (vph) | 0 | 31 | 41 | 0 | 30 | 0 | 102 | 670 | 22 | 11 | 2083 | 16 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Actuated Green, G (s) | 8.9 | 8.9 | 8.9 | 8.9 | 10.3 | 87.9 | 87.9 | 1.2 | 78.8 | 78.8 | | |
| Effective Green, g (s) | 8.9 | 8.9 | 8.9 | 8.9 | 10.3 | 87.9 | 87.9 | 1.2 | 78.8 | 78.8 | | |
| Actuated g/C Ratio | 0.08 | 0.08 | 0.08 | 0.08 | 0.09 | 0.80 | 0.80 | 0.01 | 0.72 | 0.72 | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | |
| Lane Grp Cap (vph) | 111 | 128 | 119 | 166 | 2828 | 1265 | 19 | 2535 | 1134 | | | |
| v/s Ratio Prot | | | | | c0.06 | 0.19 | | 0.01 | c0.59 | | | |
| v/s Ratio Perm | 0.02 | 0.09 | 0.04 | | | 0.02 | | | | | | 0.01 |
| v/c Ratio | 0.28 | 0.32 | 0.26 | 0.61 | 0.24 | 0.02 | 0.58 | 0.82 | 0.01 | | | |
| Uniform Delay, d1 | 47.5 | 47.7 | 47.4 | 47.9 | 2.7 | 2.3 | 54.1 | 10.8 | 4.5 | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.27 | 0.24 | 0.05 | | | |
| Incremental Delay, d2 | 1.4 | 1.5 | 1.1 | 6.6 | 0.2 | 0.0 | 27.2 | 2.3 | 0.0 | | | |
| Delay (s) | 48.9 | 49.2 | 48.6 | 54.5 | 2.9 | 2.3 | 96.1 | 4.9 | 0.2 | | | |
| Level of Service | D | D | D | D | A | A | F | A | A | | | |
| Approach Delay (s) | 49.1 | | 48.6 | | 9.5 | | 5.3 | | | | | |
| Approach LOS | D | | D | | A | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | | 9.6 | | HCM Level of Service | A | | | | | | |
| HCM Volume to Capacity ratio | | | 0.83 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 12.0 | | | | | | |
| Intersection Capacity Utilization | | | 77.9% | | ICU Level of Service | D | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|-------|----------------------|-------|-------|------|------|------|------|-------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.96 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1787 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1787 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 92 | 54 | 20 | 170 | 72 | 32 | 108 | 406 | 125 | 148 | 1449 | 728 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 100 | 59 | 22 | 185 | 78 | 35 | 117 | 441 | 136 | 161 | 1575 | 791 |
| RTOR Reduction (vph) | 0 | 14 | 0 | 0 | 0 | 31 | 0 | 0 | 68 | 0 | 0 | 295 |
| Lane Group Flow (vph) | 100 | 67 | 0 | 128 | 135 | 4 | 117 | 441 | 68 | 161 | 1575 | 496 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | 8 | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 9.4 | 9.4 | | 13.5 | 13.5 | 13.5 | 9.6 | 53.5 | 53.5 | 14.6 | 58.5 | 58.5 |
| Effective Green, g (s) | 9.4 | 9.4 | | 13.5 | 13.5 | 13.5 | 9.6 | 53.5 | 53.5 | 14.6 | 58.5 | 58.5 |
| Actuated g/C Ratio | 0.09 | 0.09 | | 0.13 | 0.13 | 0.13 | 0.09 | 0.50 | 0.50 | 0.14 | 0.55 | 0.55 |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 302 | 157 | | 212 | 219 | 200 | 159 | 1770 | 792 | 242 | 1935 | 865 |
| v/s Ratio Prot | 0.03 | c0.05 | | 0.08 | c0.08 | | 0.07 | 0.12 | | c0.09 | 0.45 | |
| v/s Ratio Perm | | | | | 0.02 | | | 0.09 | | | | 0.50 |
| v/c Ratio | 0.33 | 0.43 | | 0.60 | 0.62 | 0.02 | 0.74 | 0.25 | 0.09 | 0.67 | 0.81 | 0.57 |
| Uniform Delay, d1 | 45.8 | 46.3 | | 44.2 | 44.3 | 41.0 | 47.5 | 15.3 | 14.0 | 43.9 | 19.8 | 16.0 |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.6 | 1.9 | | 4.8 | 5.1 | 0.0 | 16.2 | 0.3 | 0.2 | 6.7 | 3.9 | 2.8 |
| Delay (s) | 46.5 | 48.1 | | 49.0 | 49.4 | 41.0 | 63.6 | 15.6 | 14.2 | 50.6 | 23.7 | 18.8 |
| Level of Service | D | D | | D | D | D | E | B | B | D | C | B |
| Approach Delay (s) | 47.2 | | | 48.2 | | | 23.4 | | | 23.9 | | |
| Approach LOS | D | | | D | | | C | | | C | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 26.9 | | HCM Level of Service | | | | C | | | | | |
| HCM Volume to Capacity ratio | 0.79 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 107.0 | | Sum of lost time (s) | | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 69.3% | | ICU Level of Service | | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|-------|------|------|-------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 218 | 216 | 60 | 0 | 0 | 0 | 224 | 722 | 396 | 0 | 517 | 544 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 237 | 235 | 65 | 0 | 0 | 0 | 243 | 785 | 430 | 0 | 562 | 591 |
| RTOR Reduction (vph) | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 82 | 0 | 0 | 324 |
| Lane Group Flow (vph) | 237 | 274 | 0 | 0 | 0 | 0 | 243 | 785 | 348 | 0 | 562 | 267 |
| Turn Type | Prot | | | Prot | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 7 | 4 | | 5 | | 2 | 6 | | 6 | | | |
| Permitted Phases | | | 2 | | 2 | | 2 | | 6 | | | |
| Actuated Green, G (s) | 19.3 | 19.3 | | 29.0 | 82.7 | 82.7 | 29.0 | 82.7 | 82.7 | 49.7 | 49.7 | 49.7 |
| Effective Green, g (s) | 19.3 | 19.3 | | 29.0 | 82.7 | 82.7 | 29.0 | 82.7 | 82.7 | 49.7 | 49.7 | 49.7 |
| Actuated g/C Ratio | 0.18 | 0.18 | | 0.26 | 0.75 | 0.75 | 0.26 | 0.75 | 0.75 | 0.45 | 0.45 | 0.45 |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 311 | 601 | | 467 | 2661 | 1190 | 467 | 2661 | 1190 | 1599 | 715 | 715 |
| v/s Ratio Prot | c0.13 | 0.09 | | c0.14 | 0.22 | | c0.14 | 0.22 | | 0.16 | | |
| v/s Ratio Perm | | | | | 0.27 | | | 0.27 | | | 0.37 | |
| v/c Ratio | 0.76 | 0.46 | | 0.52 | 0.30 | 0.29 | 0.52 | 0.30 | 0.29 | 0.35 | 0.37 | |
| Uniform Delay, d1 | 43.2 | 40.6 | | 34.6 | 4.4 | 4.3 | 34.6 | 4.4 | 4.3 | 19.6 | 19.9 | |
| Progression Factor | 1.00 | 1.00 | | 0.99 | 0.76 | 0.39 | 0.99 | 0.76 | 0.39 | 1.00 | 1.00 | |
| Incremental Delay, d2 | 10.5 | 0.6 | | 1.0 | 0.3 | 0.6 | 1.0 | 0.3 | 0.6 | 0.6 | 1.5 | |
| Delay (s) | 53.7 | 41.2 | | 35.2 | 3.6 | 2.3 | 35.2 | 3.6 | 2.3 | 20.3 | 21.4 | |
| Level of Service | D | D | | D | A | A | D | A | A | C | C | |
| Approach Delay (s) | 46.7 | | | 0.0 | | | 8.5 | | | 20.8 | | |
| Approach LOS | D | | | A | | | A | | | C | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 19.5 | | HCM Level of Service | | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.72 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 52.8% | | ICU Level of Service | | | | A | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Satd. Flow (prot) | 1833 | 1583 | 1770 | 1583 | 1583 | 3529 | 3529 | 1770 | 3539 | 1770 | 3539 | 1770 |
| Flt Permitted | 0.98 | 1.00 | 0.72 | 1.00 | 1.00 | 1.00 | 1.00 | 0.22 | 1.00 | 0.22 | 1.00 | 0.95 |
| Satd. Flow (perm) | 1833 | 1583 | 1347 | 1583 | 1583 | 3529 | 3529 | 418 | 3539 | 418 | 3539 | 1770 |
| Volume (vph) | 16 | 32 | 132 | 12 | 0 | 136 | 0 | 1071 | 20 | 80 | 470 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 17 | 35 | 143 | 13 | 0 | 148 | 0 | 1164 | 22 | 87 | 511 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 131 | 0 | 0 | 85 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 52 | 12 | 13 | 0 | 63 | 0 | 1186 | 0 | 87 | 511 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 2 | | | 6 | | | 6 | | |
| Permitted Phases | 4 | | | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 9.6 | | | 9.6 | | | 9.6 | | | 9.6 | | |
| Effective Green, g (s) | 9.6 | | | 9.6 | | | 9.6 | | | 9.6 | | |
| Actuated g/C Ratio | 0.09 | | | 0.09 | | | 0.09 | | | 0.09 | | |
| Clearance Time (s) | 4.0 | | | 4.0 | | | 4.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | | | 3.0 | | | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 160 | 138 | 118 | 138 | 2964 | 351 | 2973 | | | | | |
| v/s Ratio Prot | 0.03 | | | 0.09 | | | 0.01 | | | 0.09 | | |
| v/c Ratio | 0.33 | | | 0.09 | | | 0.11 | | | 0.46 | | |
| Uniform Delay, d1 | 47.2 | | | 46.2 | | | 46.3 | | | 47.7 | | |
| Progression Factor | 1.00 | | | 1.00 | | | 1.00 | | | 1.00 | | |
| Incremental Delay, d2 | 1.2 | | | 0.3 | | | 0.4 | | | 2.4 | | |
| Delay (s) | 48.3 | | | 46.5 | | | 46.7 | | | 50.1 | | |
| Level of Service | D | | | D | | | D | | | A | | |
| Approach Delay (s) | 47.0 | | | 49.8 | | | 2.5 | | | 1.4 | | |
| Approach LOS | D | | | D | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 9.8 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.46 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 53.9% | | | ICU Level of Service | | | A | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Baseline AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 26 | 200 | 48 | 172 | 645 | 36 | 85 | 12 | 76 | 28 | 8 | 52 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 28 | 217 | 52 | 187 | 701 | 39 | 92 | 13 | 83 | 30 | 9 | 57 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 740 | | | 270 | | | 1436 | | | 1414 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 740 | | | 270 | | | 1436 | | | 1414 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | |
| p0 queue free % | 97 | | | 86 | | | 0 | | | 89 | | |
| cM capacity (veh/h) | 866 | | | 1294 | | | 79 | | | 114 | | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 28 | 270 | 187 | 740 | 105 | 83 | 96 | | | | | |
| Volume Left | 28 | 0 | 187 | 0 | 92 | 0 | 30 | | | | | |
| Volume Right | 0 | 52 | 0 | 39 | 0 | 83 | 57 | | | | | |
| cSH | 866 | 1700 | 1294 | 1700 | 82 | 795 | 171 | | | | | |
| Volume to Capacity | 0.03 | 0.16 | 0.14 | 0.44 | 1.28 | 0.10 | 0.56 | | | | | |
| Queue Length (ft) | 3 | 0 | 13 | 0 | 198 | 9 | 73 | | | | | |
| Control Delay (s) | 9.3 | 0.0 | 8.3 | 0.0 | 284.5 | 10.1 | 49.7 | | | | | |
| Lane LOS | A | A | A | F | B | E | E | | | | | |
| Approach Delay (s) | 0.9 | | | 1.7 | | | 163.9 | | | 49.7 | | |
| Approach LOS | D | | | D | | | F | | | E | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 24.8 | | | | | | | | | | | |
| Intersection Capacity Utilization | 61.5% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | |
| Frt | 0.90 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1653 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Flt Permitted | 0.96 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1602 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Volume (vph) | 6 | 0 | 19 | 0 | 0 | 0 | 18 | 1993 | 0 | 0 | 760 | 17 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 7 | 0 | 21 | 0 | 0 | 0 | 20 | 2166 | 0 | 0 | 826 | 18 |
| RTOR Reduction (vph) | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 10 | 0 | 0 | 0 | 0 | 20 | 2166 | 0 | 0 | 843 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Effective Green, g (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Actuated g/C Ratio | 0.16 | | 0.03 | | 0.76 | | 0.70 | | 0.70 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 262 | | 45 | | 2703 | | 2476 | | 2476 | | 2476 | |
| v/s Ratio Prot | 0.01 | | c0.61 | | 0.24 | | 0.24 | | 0.24 | | 0.24 | |
| v/s Ratio Perm | c0.02 | | 0.44 | | 0.80 | | 0.34 | | 0.34 | | 0.34 | |
| v/c Ratio | 0.04 | | 0.06 | | 0.01 | | 0.16 | | 0.82 | | 0.02 | |
| Uniform Delay, d1 | 38.7 | | 39.9 | | 39.5 | | 53.9 | | 10.2 | | 4.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.08 | | 0.81 | | 0.89 | | 1.10 | |
| Incremental Delay, d2 | 0.1 | | 0.1 | | 0.0 | | 2.8 | | 2.2 | | 0.0 | |
| Delay (s) | 38.8 | | 40.0 | | 39.6 | | 60.7 | | 10.5 | | 3.7 | |
| Level of Service | D | | D | | E | | B | | A | | F | |
| Approach Delay (s) | 38.8 | | 39.7 | | 39.7 | | 10.5 | | 5.3 | | 5.3 | |
| Approach LOS | D | | D | | D | | B | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.7 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.68 | | HCM Level of Service | | A | | A | | A | | A | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Intersection Capacity Utilization | 65.1% | | ICU Level of Service | | C | | C | | C | | C | |
| Analysis Period (min) | 15 | | ICU Level of Service | | C | | C | | C | | C | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Frt | 0.96 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.96 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (prot) | 1731 | | 1770 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Flt Permitted | 0.88 | | 0.75 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (perm) | 1580 | | 1398 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Volume (vph) | 7 | 0 | 3 | 12 | 0 | 16 | 3 | 1947 | 21 | 11 | 741 | 4 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 8 | 0 | 3 | 13 | 0 | 17 | 3 | 2116 | 23 | 12 | 805 | 4 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 14 | 0 | 0 | 5 | 0 | 0 | 1 |
| Lane Group Flow (vph) | 0 | 8 | 0 | 0 | 13 | 3 | 3 | 2116 | 18 | 12 | 805 | 3 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.01 | | 0.73 | | 0.73 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 241 | | 214 | | 242 | | 19 | | 2574 | | 1151 | |
| v/s Ratio Prot | 0.01 | | 0.01 | | 0.01 | | 0.00 | | c0.60 | | c0.01 | |
| v/s Ratio Perm | 0.01 | | 0.06 | | 0.01 | | 0.16 | | 0.82 | | 0.02 | |
| v/c Ratio | 0.04 | | 0.06 | | 0.01 | | 0.16 | | 0.82 | | 0.02 | |
| Uniform Delay, d1 | 39.7 | | 39.9 | | 39.5 | | 53.9 | | 10.2 | | 4.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.08 | | 0.81 | | 0.89 | | 1.10 | |
| Incremental Delay, d2 | 0.1 | | 0.1 | | 0.0 | | 2.8 | | 2.2 | | 0.0 | |
| Delay (s) | 39.8 | | 40.0 | | 39.6 | | 60.7 | | 10.5 | | 3.7 | |
| Level of Service | D | | D | | E | | B | | A | | F | |
| Approach Delay (s) | 39.8 | | 39.7 | | 39.7 | | 10.5 | | 5.3 | | 5.3 | |
| Approach LOS | D | | D | | D | | B | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 9.5 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.69 | | HCM Level of Service | | A | | A | | A | | A | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Intersection Capacity Utilization | 70.5% | | ICU Level of Service | | C | | C | | C | | C | |
| Analysis Period (min) | 15 | | ICU Level of Service | | C | | C | | C | | C | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|-------|------|----------------------|-------|------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 20 | 103 | 182 | 1957 | 754 | 26 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 22 | 112 | 198 | 2127 | 820 | 28 |
| RTOR Reduction (vph) | 0 | 105 | 0 | 0 | 0 | 10 |
| Lane Group Flow (vph) | 22 | 7 | 198 | 2127 | 820 | 18 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | | 2 | |
| Permitted Phases | 4 | | 6 | | 6 | |
| Actuated Green, G (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Effective Green, g (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Actuated g/C Ratio | 0.07 | 0.07 | 0.18 | 0.86 | 0.65 | 0.65 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 117 | 105 | 317 | 3047 | 2284 | 1022 |
| v/s Ratio Prot | 0.01 | | 0.11 | c0.60 | 0.23 | |
| v/s Ratio Perm | | 0.07 | | | | 0.02 |
| v/c Ratio | 0.19 | 0.07 | 0.62 | 0.70 | 0.36 | 0.02 |
| Uniform Delay, d1 | 48.5 | 48.2 | 41.7 | 2.7 | 9.0 | 7.0 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.35 | 2.74 |
| Incremental Delay, d2 | 0.8 | 0.3 | 3.8 | 1.4 | 0.4 | 0.0 |
| Delay (s) | 49.3 | 48.5 | 45.5 | 4.0 | 12.5 | 19.2 |
| Level of Service | D | D | D | A | B | B |
| Approach Delay (s) | 48.6 | | 7.6 | | 12.8 | |
| Approach LOS | D | | A | | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | 10.6 | | HCM Level of Service | | B | |
| HCM Volume to Capacity ratio | 0.72 | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | |
| Intersection Capacity Utilization | 64.1% | | ICU Level of Service | | C | |
| Analysis Period (min) | 15 | | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | |
| Flt Protected | 0.96 | 1.00 | | 0.96 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1781 | 1583 | | 1739 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Flt Permitted | 0.80 | 1.00 | | 0.76 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1483 | 1583 | | 1371 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Volume (vph) | 19 | 2 | 87 | 22 | 0 | 6 | 123 | 2118 | 33 | 14 | 888 | 24 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 21 | 2 | 95 | 24 | 0 | 7 | 134 | 2302 | 36 | 15 | 965 | 26 |
| RTOR Reduction (vph) | 0 | 0 | 90 | 0 | 7 | 0 | 0 | 0 | 5 | 0 | 0 | 7 |
| Lane Group Flow (vph) | 0 | 23 | 5 | 0 | 24 | 0 | 134 | 2302 | 31 | 15 | 965 | 19 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Actuated Green, G (s) | 8.1 | | 8.1 | | 8.1 | | 25.4 | | 126.7 | | 104.5 | |
| Effective Green, g (s) | 8.1 | | 8.1 | | 8.1 | | 25.4 | | 126.7 | | 104.5 | |
| Actuated g/C Ratio | 0.05 | | 0.05 | | 0.05 | | 0.17 | | 0.84 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 80 | | 85 | | 74 | | 300 | | 2989 | | 1103 | |
| v/s Ratio Prot | | | | | | | 0.08 | | c0.65 | | 0.01 | |
| v/s Ratio Perm | 0.02 | | 0.06 | | 0.02 | | | | 0.02 | | 0.02 | |
| v/c Ratio | 0.29 | | 0.06 | | 0.33 | | 0.45 | | 0.77 | | 0.39 | |
| Uniform Delay, d1 | 68.2 | | 67.3 | | 68.3 | | 56.0 | | 5.2 | | 1.8 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 0.77 | | 0.06 | | 0.00 | |
| Incremental Delay, d2 | 2.0 | | 0.3 | | 2.6 | | 0.4 | | 0.7 | | 0.0 | |
| Delay (s) | 70.2 | | 67.6 | | 70.9 | | 43.7 | | 1.0 | | 79.1 | |
| Level of Service | E | | E | | E | | D | | A | | A | |
| Approach Delay (s) | 68.1 | | 70.9 | | 3.3 | | 10.9 | | | | | |
| Approach LOS | E | | E | | A | | B | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.1 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.79 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 80.1% | | | ICU Level of Service | | | D | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|------|-------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.90 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 502 | 64 | 112 | 103 | 61 | 112 | 365 | 1782 | 72 | 86 | 761 | 94 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 546 | 70 | 122 | 112 | 66 | 122 | 397 | 1937 | 78 | 93 | 827 | 102 |
| RTOR Reduction (vph) | 0 | 42 | 0 | 0 | 0 | 95 | 0 | 0 | 19 | 0 | 0 | 60 |
| Lane Group Flow (vph) | 546 | 150 | 0 | 87 | 91 | 27 | 397 | 1937 | 59 | 93 | 827 | 42 |
| Turn Type | Split | | Split | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | 4 | 8 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | 8 | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 22.0 | 22.0 | 12.6 | 12.6 | 12.6 | 47.0 | 88.3 | 88.3 | 11.1 | 52.4 | 52.4 | |
| Effective Green, g (s) | 22.0 | 22.0 | 12.6 | 12.6 | 12.6 | 47.0 | 88.3 | 88.3 | 11.1 | 52.4 | 52.4 | |
| Actuated g/C Ratio | 0.15 | 0.15 | 0.08 | 0.08 | 0.08 | 0.31 | 0.59 | 0.59 | 0.07 | 0.35 | 0.35 | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 504 | 247 | 141 | 147 | 133 | 555 | 2083 | 932 | 131 | 1236 | 553 | |
| v/s Ratio Prot | c0.16 | 0.11 | 0.05 | 0.05 | | 0.22 | c0.55 | | 0.05 | c0.23 | | |
| v/s Ratio Perm | | | | | 0.08 | | | 0.05 | | | | 0.06 |
| v/c Ratio | 1.08 | 0.61 | 0.62 | 0.62 | 0.20 | 0.72 | 0.93 | 0.06 | 0.71 | 0.67 | 0.08 | |
| Uniform Delay, d1 | 64.0 | 60.0 | 66.4 | 66.4 | 64.0 | 45.6 | 28.0 | 13.2 | 67.9 | 41.4 | 32.6 | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.92 | 0.85 | 1.14 | 0.88 | 0.83 | 1.41 | |
| Incremental Delay, d2 | 64.5 | 4.2 | 7.8 | 7.5 | 0.7 | 4.2 | 8.7 | 0.1 | 15.4 | 2.7 | 0.3 | |
| Delay (s) | 128.5 | 64.2 | 74.2 | 73.9 | 64.8 | 46.1 | 32.6 | 15.1 | 75.2 | 37.2 | 46.4 | |
| Level of Service | F | E | E | E | E | D | C | B | E | D | D | |
| Approach Delay (s) | 111.8 | | 70.3 | | 34.3 | | 41.5 | | | | | |
| Approach LOS | F | | E | | C | | D | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 51.1 | | HCM Level of Service | | D | | | | | | | |
| HCM Volume to Capacity ratio | 0.94 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 16.0 | | | | | | | |
| Intersection Capacity Utilization | 86.1% | | ICU Level of Service | | E | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|-------|------|-------|-------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.93 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 401 | 241 | 192 | 0 | 0 | 0 | 207 | 961 | 411 | 0 | 635 | 351 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 436 | 262 | 209 | 0 | 0 | 0 | 225 | 1045 | 447 | 0 | 690 | 382 |
| RTOR Reduction (vph) | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 143 | 0 | 0 | 227 |
| Lane Group Flow (vph) | 436 | 356 | 0 | 0 | 0 | 0 | 225 | 1045 | 304 | 0 | 690 | 155 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | | | 5 | | 2 | | | 6 | | |
| Permitted Phases | | | 2 | | 2 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 40.1 | 40.1 | | | 37.0 | | 101.9 | 101.9 | 60.9 | | 60.9 | |
| Effective Green, g (s) | 40.1 | 40.1 | | | 37.0 | | 101.9 | 101.9 | 60.9 | | 60.9 | |
| Actuated g/C Ratio | 0.27 | 0.27 | | | 0.25 | | 0.68 | 0.68 | 0.41 | | 0.41 | |
| Clearance Time (s) | 4.0 | 4.0 | | | 4.0 | | 4.0 | 4.0 | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | 3.0 | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 473 | 883 | | | 437 | | 2404 | 1075 | 1437 | | 643 | |
| v/s Ratio Prot | c0.25 | 0.14 | | | c0.13 | | 0.30 | | 0.19 | | | |
| v/s Ratio Perm | | | | | | | | 0.28 | | | 0.24 | |
| v/c Ratio | 0.92 | 0.40 | | | 0.51 | | 0.43 | 0.28 | 0.48 | | 0.24 | |
| Uniform Delay, d1 | 53.4 | 45.1 | | | 48.8 | | 10.9 | 9.5 | 32.9 | | 29.3 | |
| Progression Factor | 1.00 | 1.00 | | | 0.96 | | 0.83 | 0.32 | 0.25 | | 0.05 | |
| Incremental Delay, d2 | 23.5 | 0.3 | | | 0.9 | | 0.5 | 0.6 | 0.9 | | 0.7 | |
| Delay (s) | 76.9 | 45.4 | | | 47.9 | | 9.6 | 3.7 | 9.2 | | 2.1 | |
| Level of Service | E | D | | | D | | A | A | A | | A | |
| Approach Delay (s) | 60.6 | | 0.0 | | 13.1 | | 6.6 | | | | | |
| Approach LOS | E | | A | | B | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 22.9 | | HCM Level of Service | | C | | | | | | | |
| HCM Volume to Capacity ratio | 0.67 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 61.2% | | ICU Level of Service | | B | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Satd. Flow (prot) | 1831 | 1583 | 1770 | 1583 | 1583 | 3529 | 3529 | 1770 | 3539 | 1770 | 3539 | 1770 |
| Flt Permitted | 0.98 | 1.00 | 0.46 | 1.00 | 1.00 | 1.00 | 1.00 | 0.16 | 1.00 | 0.16 | 1.00 | 0.16 |
| Satd. Flow (perm) | 1831 | 1583 | 849 | 1583 | 1583 | 3529 | 3529 | 297 | 3539 | 297 | 3539 | 297 |
| Volume (vph) | 35 | 64 | 175 | 17 | 0 | 31 | 0 | 1336 | 27 | 121 | 599 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 38 | 70 | 190 | 18 | 0 | 34 | 0 | 1452 | 29 | 132 | 651 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 172 | 0 | 0 | 31 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 108 | 18 | 18 | 0 | 3 | 0 | 1480 | 0 | 132 | 651 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | 4 | 8 | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 13.9 | 13.9 | 13.9 | 13.9 | 13.9 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 |
| Effective Green, g (s) | 13.9 | 13.9 | 13.9 | 13.9 | 13.9 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 | 128.1 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.09 | 0.09 | 0.09 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 170 | 147 | 79 | 147 | 147 | 3014 | 254 | 3022 | 254 | 3022 | 254 | 3022 |
| v/s Ratio Prot | | | | | | 0.42 | | 0.18 | | | | |
| v/s Ratio Perm | 0.06 | 0.12 | 0.02 | 0.02 | 0.02 | c0.45 | | 0.45 | | | | |
| v/c Ratio | 0.64 | 0.12 | 0.23 | 0.02 | 0.02 | 0.49 | | 0.52 | | | | |
| Uniform Delay, d1 | 65.6 | 62.4 | 63.1 | 61.9 | 61.9 | 2.8 | | 2.9 | | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | 1.64 | | | | |
| Incremental Delay, d2 | 7.5 | 0.4 | 1.5 | 0.1 | 0.1 | 0.6 | | 6.8 | | | | |
| Delay (s) | 73.2 | 62.8 | 64.5 | 61.9 | 61.9 | 3.3 | | 11.5 | | | | |
| Level of Service | E | E | E | E | E | A | | B | | | | |
| Approach Delay (s) | 66.6 | | | 62.8 | | 3.3 | | 2.5 | | | | |
| Approach LOS | E | | | E | | A | | F | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 11.5 | | | HCM Level of Service | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.60 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 66.5% | | | ICU Level of Service | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Baseline PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|-------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 46 | 612 | 49 | 104 | 292 | 85 | 57 | 8 | 107 | 61 | 12 | 19 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 50 | 665 | 53 | 113 | 317 | 92 | 62 | 9 | 116 | 66 | 13 | 21 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 410 | 718 | | | 1362 | | | 1428 | 692 | 1359 | 1408 | 364 |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 410 | 718 | | | 1362 | | | 1428 | 692 | 1359 | 1408 | 364 |
| tC, single (s) | 4.1 | 4.1 | | | 7.1 | | | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | 2.2 | | | 3.5 | | | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free % | 96 | 87 | | | 36 | | | 92 | 74 | 13 | 89 | 97 |
| cM capacity (veh/h) | 1149 | 883 | | | 97 | | | 113 | 444 | 76 | 116 | 681 |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 50 | 718 | 113 | 410 | 71 | 116 | 100 | | | | | |
| Volume Left | 50 | 0 | 113 | 0 | 62 | 0 | 66 | | | | | |
| Volume Right | 0 | 53 | 0 | 92 | 0 | 116 | 21 | | | | | |
| cSH | 1149 | 1700 | 883 | 1700 | 99 | 444 | 99 | | | | | |
| Volume to Capacity | 0.04 | 0.42 | 0.13 | 0.24 | 0.72 | 0.26 | 1.01 | | | | | |
| Queue Length (ft) | 3 | 0 | 11 | 0 | 92 | 26 | 155 | | | | | |
| Control Delay (s) | 8.3 | 0.0 | 9.7 | 0.0 | 103.5 | 16.0 | 172.9 | | | | | |
| Lane LOS | A | A | A | F | C | F | F | | | | | |
| Approach Delay (s) | 0.5 | 2.1 | | 49.1 | | 172.9 | | | | | | |
| Approach LOS | E | E | | F | | F | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 17.7 | | | | | | | | | | | |
| Intersection Capacity Utilization | 62.8% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Roa

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|-------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 1.00 | | 0.95 | |
| Frt | 0.86 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Flt Permitted | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Volume (vph) | 0 | 0 | 34 | 0 | 0 | 0 | 15 | 561 | 0 | 0 | 1404 | 9 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 37 | 0 | 0 | 0 | 16 | 610 | 0 | 0 | 1526 | 10 |
| RTOR Reduction (vph) | 0 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 1 | 0 | 0 | 0 | 0 | 16 | 610 | 0 | 0 | 1536 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Effective Green, g (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Actuated g/C Ratio | 0.03 | | 0.03 | | 0.90 | | 0.83 | | 0.83 | | 0.83 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 48 | | 50 | | 3175 | | 2945 | | 2945 | | 2945 | |
| v/s Ratio Prot | c0.02 | | c0.01 | | 0.17 | | c0.43 | | c0.43 | | c0.43 | |
| v/s Ratio Perm | 0.02 | | 0.32 | | 0.19 | | 0.52 | | 0.52 | | 0.52 | |
| v/c Ratio | 0.02 | | 0.32 | | 0.19 | | 0.52 | | 0.52 | | 0.52 | |
| Uniform Delay, d1 | 51.8 | | 52.4 | | 0.7 | | 2.7 | | 2.7 | | 2.7 | |
| Progression Factor | 1.00 | | 0.81 | | 0.70 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.2 | | 3.6 | | 0.1 | | 0.7 | | 0.7 | | 0.7 | |
| Delay (s) | 52.0 | | 45.9 | | 0.6 | | 3.4 | | 3.4 | | 3.4 | |
| Level of Service | D | | D | | A | | A | | A | | A | |
| Approach Delay (s) | 52.0 | | 0.0 | | 1.8 | | 3.4 | | 3.4 | | 3.4 | |
| Approach LOS | D | | A | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 3.7 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.52 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | A | | A | | A | | A | |
| Intersection Capacity Utilization | 49.1% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Roa

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Frt | 0.90 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (prot) | 1659 | | 1770 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Flt Permitted | 0.96 | | 0.75 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (perm) | 1618 | | 1403 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Volume (vph) | 2 | 0 | 5 | 17 | 0 | 9 | 0 | 629 | 12 | 7 | 1561 | 1 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 2 | 0 | 5 | 18 | 0 | 10 | 0 | 684 | 13 | 8 | 1697 | 1 |
| RTOR Reduction (vph) | 0 | 4 | 0 | 0 | 0 | 8 | 0 | 0 | 4 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 3 | 0 | 0 | 18 | 2 | 0 | 684 | 9 | 8 | 1697 | 1 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | 6 |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.73 | | 0.73 | | 0.01 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 247 | | 214 | | 242 | | 2574 | | 1151 | | 19 | |
| v/s Ratio Prot | 0.00 | | c0.01 | | 0.01 | | 0.19 | | 0.01 | | c0.48 | |
| v/s Ratio Perm | 0.01 | | 0.08 | | 0.01 | | 0.27 | | 0.01 | | 0.62 | |
| v/c Ratio | 0.01 | | 0.08 | | 0.01 | | 0.27 | | 0.01 | | 0.62 | |
| Uniform Delay, d1 | 39.6 | | 40.0 | | 39.5 | | 5.1 | | 4.1 | | 54.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.25 | | 1.86 | | 1.22 | |
| Incremental Delay, d2 | 0.0 | | 0.2 | | 0.0 | | 0.2 | | 0.0 | | 13.2 | |
| Delay (s) | 39.6 | | 40.2 | | 39.5 | | 6.6 | | 7.7 | | 79.4 | |
| Level of Service | D | | D | | D | | A | | A | | E | |
| Approach Delay (s) | 39.6 | | 39.9 | | 6.6 | | 5.0 | | 5.0 | | 5.0 | |
| Approach LOS | D | | D | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 6.0 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.53 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | A | | A | | A | | A | |
| Intersection Capacity Utilization | 53.2% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Roa

Baseline + Project AM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 22 | 224 | 94 | 630 | 1659 | 10 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 24 | 243 | 102 | 685 | 1803 | 11 |
| RTOR Reduction (vph) | 0 | 177 | 0 | 0 | 0 | 3 |
| Lane Group Flow (vph) | 24 | 66 | 102 | 685 | 1803 | 8 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | | 2 | |
| Permitted Phases | 4 | | 6 | | 6 | |
| Actuated Green, G (s) | 10.2 | 10.2 | 11.2 | 91.8 | 76.6 | 76.6 |
| Effective Green, g (s) | 10.2 | 10.2 | 11.2 | 91.8 | 76.6 | 76.6 |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.10 | 0.83 | 0.70 | 0.70 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 164 | 147 | 180 | 2953 | 2464 | 1102 |
| v/s Ratio Prot | 0.01 | | c0.06 | | 0.19 | |
| v/s Ratio Perm | 0.15 | | 0.23 | | 0.01 | |
| v/c Ratio | 0.15 | 0.45 | 0.57 | 0.23 | 0.73 | 0.01 |
| Uniform Delay, d1 | 45.9 | 47.2 | 47.1 | 1.9 | 10.3 | 5.1 |
| Progression Factor | 1.00 | 1.00 | 0.99 | 0.93 | 0.69 | 0.92 |
| Incremental Delay, d2 | 0.4 | 2.2 | 4.0 | 0.2 | 1.6 | 0.0 |
| Delay (s) | 46.3 | 49.4 | 50.4 | 1.9 | 8.7 | 4.7 |
| Level of Service | D | D | D | A | A | A |
| Approach Delay (s) | 49.1 | | 8.2 | | 8.7 | |
| Approach LOS | D | | A | | A | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | 12.3 | | HCM Level of Service | | B | |
| HCM Volume to Capacity ratio | 0.81 | | Sum of lost time (s) | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | |
| Intersection Capacity Utilization | 66.4% | | Analysis Period (min) | | 15 | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Roa

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|-----------------------|------|------|-------|------|------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.93 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.98 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1782 | 1583 | 1698 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.74 | 1.00 | 0.85 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1378 | 1583 | 1473 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 26 | 3 | 137 | 23 | 3 | 27 | 94 | 630 | 25 | 10 | 1922 | 20 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 28 | 3 | 149 | 25 | 3 | 29 | 102 | 685 | 27 | 11 | 2089 | 22 |
| RTOR Reduction (vph) | 0 | 0 | 108 | 0 | 27 | 0 | 0 | 0 | 5 | 0 | 0 | 6 |
| Lane Group Flow (vph) | 0 | 31 | 41 | 0 | 30 | 0 | 102 | 685 | 22 | 11 | 2089 | 16 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Prot | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 6 | |
| Actuated Green, G (s) | 8.9 | 8.9 | 8.9 | 10.3 | 87.9 | 87.9 | 1.2 | 78.8 | 78.8 | 1.2 | 78.8 | 78.8 |
| Effective Green, g (s) | 8.9 | 8.9 | 8.9 | 10.3 | 87.9 | 87.9 | 1.2 | 78.8 | 78.8 | 1.2 | 78.8 | 78.8 |
| Actuated g/C Ratio | 0.08 | 0.08 | 0.08 | 0.09 | 0.80 | 0.80 | 0.01 | 0.72 | 0.72 | 0.01 | 0.72 | 0.72 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 111 | 128 | 119 | 166 | 2828 | 1265 | 19 | 2535 | 1134 | 0.01 | c0.59 | 0.01 |
| v/s Ratio Prot | 0.02 | | 0.09 | | 0.04 | | c0.06 | | 0.19 | | 0.01 | |
| v/s Ratio Perm | 0.28 | | 0.32 | | 0.26 | | 0.61 | | 0.24 | | 0.02 | |
| v/c Ratio | 0.28 | 0.32 | 0.26 | 0.61 | 0.24 | 0.02 | 0.58 | 0.82 | 0.01 | 0.02 | 0.58 | 0.82 |
| Uniform Delay, d1 | 47.5 | 47.7 | 47.4 | 47.9 | 2.8 | 2.3 | 54.1 | 10.8 | 4.5 | 1.27 | 0.24 | 0.05 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.27 | 0.24 | 0.05 | 1.4 | 1.5 | 1.1 |
| Incremental Delay, d2 | 1.4 | 1.5 | 1.1 | 6.6 | 0.2 | 0.0 | 27.1 | 2.3 | 0.0 | 48.9 | 49.2 | 48.6 |
| Delay (s) | 48.9 | 49.2 | 48.6 | 54.5 | 3.0 | 2.3 | 96.2 | 5.0 | 0.2 | D | D | D |
| Level of Service | D | D | D | D | A | A | F | A | A | D | D | D |
| Approach Delay (s) | 49.1 | | 48.6 | | 9.4 | | 5.4 | | D | | A | |
| Approach LOS | D | | D | | A | | A | | D | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 9.7 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.83 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | ICU Level of Service | | | D | | | | | |
| Intersection Capacity Utilization | 78.1% | | | Analysis Period (min) | | | 15 | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|-------|----------------------|-------|-------|------|-------|------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.91 | |
| Frt | 1.00 | 0.94 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.95 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 3433 | 1750 | | 1681 | 1734 | 1583 | 1770 | 3539 | 1583 | 1770 | 4829 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 3433 | 1750 | | 1681 | 1734 | 1583 | 1770 | 3539 | 1583 | 1770 | 4829 | |
| Volume (vph) | 107 | 63 | 42 | 172 | 74 | 32 | 108 | 406 | 125 | 148 | 1451 | 733 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 116 | 68 | 46 | 187 | 80 | 35 | 117 | 441 | 136 | 161 | 1577 | 797 |
| RTOR Reduction (vph) | 0 | 23 | 0 | 0 | 0 | 31 | 0 | 0 | 71 | 0 | 71 | 0 |
| Lane Group Flow (vph) | 116 | 91 | 0 | 130 | 137 | 4 | 117 | 441 | 65 | 161 | 2303 | 0 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | | | 8 | | | 2 | | | | |
| Actuated Green, G (s) | 10.9 | 10.9 | | 13.2 | 13.2 | 13.2 | 9.7 | 52.2 | 52.2 | 17.7 | 60.2 | |
| Effective Green, g (s) | 10.9 | 10.9 | | 13.2 | 13.2 | 13.2 | 9.7 | 52.2 | 52.2 | 17.7 | 60.2 | |
| Actuated g/C Ratio | 0.10 | 0.10 | | 0.12 | 0.12 | 0.12 | 0.09 | 0.47 | 0.47 | 0.16 | 0.55 | |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 340 | 173 | | 202 | 208 | 190 | 156 | 1679 | 751 | 285 | 2643 | |
| v/s Ratio Prot | 0.03 | c0.07 | | 0.08 | c0.08 | | c0.07 | 0.12 | | 0.09 | c0.49 | |
| v/s Ratio Perm | | | | | 0.02 | | | 0.09 | | | | |
| v/c Ratio | 0.34 | 0.52 | | 0.64 | 0.66 | 0.02 | 0.75 | 0.26 | 0.09 | 0.56 | 0.87 | |
| Uniform Delay, d1 | 46.2 | 47.1 | | 46.2 | 46.2 | 42.7 | 49.0 | 17.3 | 15.8 | 42.6 | 21.5 | |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 0.93 | 0.70 | 0.19 | 0.84 | 0.69 | |
| Incremental Delay, d2 | 0.6 | 2.8 | | 6.9 | 7.3 | 0.0 | 17.5 | 0.4 | 0.2 | 1.9 | 3.2 | |
| Delay (s) | 46.8 | 49.9 | | 53.0 | 53.6 | 42.8 | 63.1 | 12.6 | 3.2 | 37.8 | 18.0 | |
| Level of Service | D | D | | D | D | D | E | B | A | D | B | |
| Approach Delay (s) | 48.4 | | | 52.1 | | | 19.2 | | | 19.2 | | |
| Approach LOS | D | | | D | | | B | | | B | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 23.7 | | HCM Level of Service | | | | C | | | | | |
| HCM Volume to Capacity ratio | 0.82 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | | | 16.0 | | | | | |
| Intersection Capacity Utilization | 73.8% | | ICU Level of Service | | | | D | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 1770 | 3424 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 218 | 216 | 60 | 0 | 0 | 0 | 224 | 729 | 396 | 0 | 534 | 544 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 237 | 235 | 65 | 0 | 0 | 0 | 243 | 792 | 430 | 0 | 580 | 591 |
| RTOR Reduction (vph) | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 82 | 0 | 0 | 324 |
| Lane Group Flow (vph) | 237 | 274 | 0 | 0 | 0 | 0 | 243 | 792 | 348 | 0 | 580 | 267 |
| Turn Type | Prot | | | Prot | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 7 | 4 | | | | | 5 | 2 | | | 6 | |
| Permitted Phases | | | | | | | | 2 | | | | 6 |
| Actuated Green, G (s) | 19.3 | 19.3 | | | | | 29.0 | 82.7 | 82.7 | | 49.7 | 49.7 |
| Effective Green, g (s) | 19.3 | 19.3 | | | | | 29.0 | 82.7 | 82.7 | | 49.7 | 49.7 |
| Actuated g/C Ratio | 0.18 | 0.18 | | | | | 0.26 | 0.75 | 0.75 | | 0.45 | 0.45 |
| Clearance Time (s) | 4.0 | 4.0 | | | | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | | | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 311 | 601 | | | | | 467 | 2661 | 1190 | | 1599 | 715 |
| v/s Ratio Prot | c0.13 | 0.09 | | | | | c0.14 | 0.22 | | | 0.16 | |
| v/s Ratio Perm | | | | | | | | 0.27 | | | | 0.37 |
| v/c Ratio | 0.76 | 0.46 | | | | | 0.52 | 0.30 | 0.29 | | 0.36 | 0.37 |
| Uniform Delay, d1 | 43.2 | 40.6 | | | | | 34.6 | 4.4 | 4.3 | | 19.8 | 19.9 |
| Progression Factor | 1.00 | 1.00 | | | | | 0.99 | 0.76 | 0.38 | | 1.00 | 1.00 |
| Incremental Delay, d2 | 10.5 | 0.6 | | | | | 1.0 | 0.3 | 0.6 | | 0.6 | 1.5 |
| Delay (s) | 53.7 | 41.2 | | | | | 35.3 | 3.6 | 2.3 | | 20.4 | 21.4 |
| Level of Service | D | D | | | | | D | A | A | | C | C |
| Approach Delay (s) | 46.7 | | | 0.0 | | | 8.4 | | | 20.9 | | |
| Approach LOS | D | | | A | | | A | | | C | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 19.5 | | HCM Level of Service | | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.72 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 52.8% | | ICU Level of Service | | | | A | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|--------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1833 | 1583 | 1770 | 1583 | 1833 | 1583 | 3529 | 3529 | 1770 | 3529 | 1770 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.72 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.22 | 1.00 | 0.22 | 1.00 |
| Satd. Flow (perm) | 1833 | 1583 | 1347 | 1583 | 1833 | 1583 | 3529 | 3529 | 414 | 3529 | 414 | 3539 |
| Volume (vph) | 16 | 32 | 132 | 12 | 0 | 136 | 0 | 1078 | 20 | 80 | 487 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 17 | 35 | 143 | 13 | 0 | 148 | 0 | 1172 | 22 | 87 | 529 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 131 | 0 | 0 | 83 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 52 | 12 | 13 | 0 | 65 | 0 | 1194 | 0 | 87 | 529 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | 6 | | 6 | |
| Permitted Phases | 4 | | | 8 | | | 8 | | 6 | | 6 | |
| Actuated Green, G (s) | 9.6 | | | 9.6 | | | 9.6 | | 92.4 | | 92.4 | |
| Effective Green, g (s) | 9.6 | | | 9.6 | | | 9.6 | | 92.4 | | 92.4 | |
| Actuated g/C Ratio | 0.09 | | | 0.09 | | | 0.09 | | 0.84 | | 0.84 | |
| Clearance Time (s) | 4.0 | | | 4.0 | | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | | 3.0 | | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 160 | | | 138 | | | 118 | | 2964 | | 348 | |
| v/s Ratio Prot | 0.03 | | | 0.09 | | | 0.01 | | 0.09 | | 0.21 | |
| v/c Ratio | 0.33 | | | 0.09 | | | 0.11 | | 0.47 | | 0.40 | |
| Uniform Delay, d1 | 47.2 | | | 46.2 | | | 46.3 | | 47.8 | | 2.1 | |
| Progression Factor | 1.00 | | | 1.00 | | | 1.00 | | 1.00 | | 1.85 | |
| Incremental Delay, d2 | 1.2 | | | 0.3 | | | 0.4 | | 2.5 | | 0.4 | |
| Delay (s) | 48.3 | | | 46.5 | | | 46.7 | | 50.3 | | 2.5 | |
| Level of Service | D | | | D | | | D | | A | | A | |
| Approach Delay (s) | 47.0 | | | 50.0 | | | 2.5 | | 1.3 | | 1.3 | |
| Approach LOS | D | | | D | | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 9.7 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.47 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 54.1% | | | ICU Level of Service | | | A | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Baseline + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | Stop | | Stop | |
| Grade | 0% | | | 0% | | | 0% | | 0% | | 0% | |
| Volume (veh/h) | 26 | 207 | 48 | 172 | 665 | 36 | 85 | 12 | 76 | 28 | 8 | 52 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 28 | 225 | 52 | 187 | 723 | 39 | 92 | 13 | 83 | 30 | 9 | 57 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 762 | | | 277 | | | 1465 | | 1443 | | 251 | |
| vC1, stage 1 conf vol | 762 | | | 277 | | | 1465 | | 1443 | | 251 | |
| vC2, stage 2 conf vol | 762 | | | 277 | | | 1465 | | 1443 | | 251 | |
| vCu, unblocked vol | 4.1 | | | 4.1 | | | 7.1 | | 6.5 | | 6.2 | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | 6.5 | | 6.2 | |
| tC, 2 stage (s) | 2.2 | | | 2.2 | | | 3.5 | | 4.0 | | 3.3 | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | 4.0 | | 3.3 | |
| p0 queue free % | 97 | | | 85 | | | 0 | | 88 | | 90 | |
| cM capacity (veh/h) | 850 | | | 1286 | | | 75 | | 109 | | 788 | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 28 | 277 | 187 | 762 | 105 | 83 | 96 | | | | | |
| Volume Left | 28 | 0 | 187 | 0 | 92 | 0 | 30 | | | | | |
| Volume Right | 0 | 52 | 0 | 39 | 0 | 83 | 57 | | | | | |
| cSH | 850 | 1700 | 1286 | 1700 | 78 | 788 | 164 | | | | | |
| Volume to Capacity | 0.03 | 0.16 | 0.15 | 0.45 | 1.36 | 0.10 | 0.58 | | | | | |
| Queue Length (ft) | 3 | 0 | 13 | 0 | 206 | 9 | 77 | | | | | |
| Control Delay (s) | 9.4 | 0.0 | 8.3 | 0.0 | 317.6 | 10.1 | 54.1 | | | | | |
| Lane LOS | A | A | A | A | F | B | F | | | | | |
| Approach Delay (s) | 0.9 | | | 1.6 | | | 182.5 | | 54.1 | | 54.1 | |
| Approach LOS | A | | | A | | | F | | F | | F | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 26.9 | | | | | | | | | | | |
| Intersection Capacity Utilization | 62.5% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Roa

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|-------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | | ↔ | | | ↕ | | | ↕ | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 1.00 | | 1.00 | |
| Flt | 0.90 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1653 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Flt Permitted | 0.96 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1602 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Volume (vph) | 6 | 0 | 19 | 0 | 0 | 0 | 18 | 2001 | 0 | 0 | 774 | 17 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 7 | 0 | 21 | 0 | 0 | 0 | 20 | 2175 | 0 | 0 | 841 | 18 |
| RTOR Reduction (vph) | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 10 | 0 | 0 | 0 | 0 | 20 | 2175 | 0 | 0 | 858 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | | |
| Permitted Phases | | | | | | | | | | | | |
| Actuated Green, G (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Effective Green, g (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Actuated g/C Ratio | 0.16 | | 0.03 | | 0.76 | | 0.70 | | 0.70 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 262 | | 45 | | 2703 | | 2476 | | 2476 | | 2476 | |
| v/s Ratio Prot | | | 0.01 | | c0.61 | | 0.24 | | 0.24 | | 0.24 | |
| v/s Ratio Perm | c0.02 | | | | | | | | | | | |
| v/c Ratio | 0.04 | | 0.44 | | 0.80 | | 0.35 | | 0.35 | | 0.35 | |
| Uniform Delay, d1 | 38.7 | | 52.8 | | 8.0 | | 6.5 | | 6.5 | | 6.5 | |
| Progression Factor | 1.00 | | 0.94 | | 0.88 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.1 | | 4.4 | | 1.7 | | 0.4 | | 0.4 | | 0.4 | |
| Delay (s) | 38.8 | | 54.3 | | 8.7 | | 6.8 | | 6.8 | | 6.8 | |
| Level of Service | D | | D | | A | | A | | A | | A | |
| Approach Delay (s) | 38.8 | | 0.0 | | 9.1 | | 6.8 | | 6.8 | | 6.8 | |
| Approach LOS | D | | A | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.8 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.68 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | | C | | C | | C | |
| Intersection Capacity Utilization | 65.3% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Roa

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | | ↔ | | | ↕ | | | ↕ | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | |
| Flt | 0.96 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.96 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1731 | | 1770 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Flt Permitted | 0.88 | | 0.75 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (perm) | 1580 | | 1398 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Volume (vph) | 7 | 0 | 3 | 12 | 0 | 16 | 3 | 1955 | 21 | 11 | 755 | 4 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 8 | 0 | 3 | 13 | 0 | 17 | 3 | 2125 | 23 | 12 | 821 | 4 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 14 | 0 | 0 | 5 | 0 | 0 | 1 |
| Lane Group Flow (vph) | 0 | 8 | 0 | 0 | 13 | 3 | 3 | 2125 | 18 | 12 | 821 | 3 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 80.0 | | 80.0 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.01 | | 0.73 | | 0.73 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 241 | | 214 | | 242 | | 19 | | 2574 | | 1151 | |
| v/s Ratio Prot | | | 0.01 | | 0.01 | | 0.00 | | c0.60 | | c0.01 | |
| v/s Ratio Perm | 0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.00 | |
| v/c Ratio | 0.04 | | 0.06 | | 0.01 | | 0.16 | | 0.83 | | 0.02 | |
| Uniform Delay, d1 | 39.7 | | 39.9 | | 39.5 | | 53.9 | | 10.2 | | 4.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.08 | | 0.81 | | 0.89 | |
| Incremental Delay, d2 | 0.1 | | 0.1 | | 0.0 | | 2.7 | | 2.3 | | 0.0 | |
| Delay (s) | 39.8 | | 40.0 | | 39.6 | | 60.8 | | 10.6 | | 3.7 | |
| Level of Service | D | | D | | E | | B | | A | | F | |
| Approach Delay (s) | 39.8 | | 39.7 | | 10.5 | | 5.3 | | 5.3 | | 5.3 | |
| Approach LOS | D | | D | | B | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 9.5 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.69 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | | C | | C | | C | |
| Intersection Capacity Utilization | 70.7% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Baseline + Project PM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|-------|------|----------------------|-------|------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 20 | 103 | 182 | 1965 | 768 | 26 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 22 | 112 | 198 | 2136 | 835 | 28 |
| RTOR Reduction (vph) | 0 | 105 | 0 | 0 | 0 | 10 |
| Lane Group Flow (vph) | 22 | 7 | 198 | 2136 | 835 | 18 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | 4 | | | | | |
| Actuated Green, G (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Effective Green, g (s) | 7.3 | 7.3 | 19.7 | 94.7 | 71.0 | 71.0 |
| Actuated g/C Ratio | 0.07 | 0.07 | 0.18 | 0.86 | 0.65 | 0.65 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 117 | 105 | 317 | 3047 | 2284 | 1022 |
| v/s Ratio Prot | 0.01 | | 0.11 | c0.60 | 0.24 | |
| v/s Ratio Perm | | 0.07 | | | | 0.02 |
| v/c Ratio | 0.19 | 0.07 | 0.62 | 0.70 | 0.37 | 0.02 |
| Uniform Delay, d1 | 48.5 | 48.2 | 41.7 | 2.7 | 9.0 | 7.0 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.35 | 2.76 |
| Incremental Delay, d2 | 0.8 | 0.3 | 3.8 | 1.4 | 0.4 | 0.0 |
| Delay (s) | 49.3 | 48.5 | 45.5 | 4.1 | 12.7 | 19.3 |
| Level of Service | D | D | D | A | B | B |
| Approach Delay (s) | 48.6 | | | 7.6 | 12.9 | |
| Approach LOS | D | | | A | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | 10.6 | | HCM Level of Service | | B | |
| HCM Volume to Capacity ratio | 0.73 | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | |
| Intersection Capacity Utilization | 64.3% | | ICU Level of Service | | C | |
| Analysis Period (min) | 15 | | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | |
| Flt Protected | 0.96 | 1.00 | | 0.96 | | 0.95 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1781 | 1583 | | 1739 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Flt Permitted | 0.80 | 1.00 | | 0.76 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (perm) | 1483 | 1583 | | 1371 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1583 |
| Volume (vph) | 19 | 2 | 87 | 22 | 0 | 6 | 123 | 2126 | 33 | 14 | 902 | 24 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 21 | 2 | 95 | 24 | 0 | 7 | 134 | 2311 | 36 | 15 | 980 | 26 |
| RTOR Reduction (vph) | 0 | 0 | 90 | 0 | 7 | 0 | 0 | 0 | 5 | 0 | 0 | 7 |
| Lane Group Flow (vph) | 0 | 23 | 5 | 0 | 24 | 0 | 134 | 2311 | 31 | 15 | 980 | 19 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Actuated Green, G (s) | 8.1 | | 8.1 | | 8.1 | | 25.4 | | 126.7 | | 104.5 | |
| Effective Green, g (s) | 8.1 | | 8.1 | | 8.1 | | 25.4 | | 126.7 | | 104.5 | |
| Actuated g/C Ratio | 0.05 | | 0.05 | | 0.05 | | 0.17 | | 0.84 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 80 | | 85 | | 74 | | 300 | | 2989 | | 1103 | |
| v/s Ratio Prot | | | | | | | 0.08 | | c0.65 | | 0.01 | |
| v/s Ratio Perm | 0.02 | | 0.06 | | 0.02 | | | | 0.02 | | 0.02 | |
| v/c Ratio | 0.29 | | 0.06 | | 0.33 | | 0.45 | | 0.77 | | 0.39 | |
| Uniform Delay, d1 | 68.2 | | 67.3 | | 68.3 | | 56.0 | | 5.2 | | 1.8 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 0.77 | | 0.06 | | 0.00 | |
| Incremental Delay, d2 | 2.0 | | 0.3 | | 2.6 | | 0.4 | | 0.7 | | 0.0 | |
| Delay (s) | 70.2 | | 67.6 | | 70.9 | | 43.5 | | 1.0 | | 79.1 | |
| Level of Service | E | | E | | E | | D | | A | | A | |
| Approach Delay (s) | 68.1 | | | | 70.9 | | 3.3 | | | | 11.0 | |
| Approach LOS | E | | | | E | | A | | | | B | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.1 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.79 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 80.4% | | | ICU Level of Service | | | D | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|------|-------|-------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | | ↔ | | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.91 | |
| Frt | 1.00 | 0.90 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.98 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 3433 | 1678 | | 1681 | 1748 | 1583 | 1770 | 3539 | 1583 | 1770 | 4996 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 3433 | 1678 | | 1681 | 1748 | 1583 | 1770 | 3539 | 1583 | 1770 | 4996 | |
| Volume (vph) | 509 | 70 | 136 | 106 | 67 | 112 | 365 | 1782 | 72 | 86 | 768 | 102 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 553 | 76 | 148 | 115 | 73 | 122 | 397 | 1937 | 78 | 93 | 835 | 111 |
| RTOR Reduction (vph) | 0 | 47 | 0 | 0 | 0 | 79 | 0 | 0 | 19 | 0 | 11 | 0 |
| Lane Group Flow (vph) | 553 | 177 | 0 | 91 | 97 | 43 | 397 | 1937 | 59 | 93 | 935 | 0 |
| Turn Type | Split | | Split | | Perm | | Prot | | Perm | | Prot | |
| Protected Phases | 4 | 4 | 8 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | 8 | | | | 2 | | | | 6 | |
| Actuated Green, G (s) | 25.0 | 25.0 | 13.0 | | 13.0 | 47.0 | | 88.0 | 8.0 | | 49.0 | |
| Effective Green, g (s) | 25.0 | 25.0 | 13.0 | | 13.0 | 47.0 | | 88.0 | 8.0 | | 49.0 | |
| Actuated g/C Ratio | 0.17 | 0.17 | 0.09 | | 0.09 | 0.31 | | 0.59 | 0.05 | | 0.33 | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | | 4.0 | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 572 | 280 | 146 | | 151 | 555 | | 2076 | 929 | | 1632 | |
| v/s Ratio Prot | c0.16 | 0.13 | 0.05 | | 0.06 | 0.22 | | c0.55 | c0.05 | | 0.19 | |
| v/s Ratio Perm | | | 0.08 | | | | 0.05 | | | | | |
| v/c Ratio | 0.97 | 0.63 | 0.62 | | 0.64 | 0.72 | | 0.93 | 0.06 | | 0.57 | |
| Uniform Delay, d1 | 62.1 | 58.2 | 66.1 | | 66.3 | 45.6 | | 28.3 | 13.3 | | 41.8 | |
| Progression Factor | 1.00 | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 29.1 | 4.6 | 8.0 | | 9.0 | 4.4 | | 9.2 | 0.1 | | 88.5 | 1.5 |
| Delay (s) | 91.2 | 62.9 | 74.2 | | 75.3 | 49.9 | | 37.6 | 13.4 | | 159.4 | 43.3 |
| Level of Service | F | E | E | | E | E | | D | D | | F | D |
| Approach Delay (s) | 83.0 | | 71.2 | | 38.8 | | 53.7 | | | | | |
| Approach LOS | F | | E | | D | | D | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 52.0 | | HCM Level of Service | | D | | | | | | | |
| HCM Volume to Capacity ratio | 0.94 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 16.0 | | | | | | | |
| Intersection Capacity Utilization | 86.6% | | ICU Level of Service | | E | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|-------|------|-------|-------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | | ↔ | | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.93 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4996 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4996 | |
| Volume (vph) | 401 | 241 | 192 | 0 | 0 | 0 | 207 | 969 | 411 | 0 | 649 | 351 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 436 | 262 | 209 | 0 | 0 | 0 | 225 | 1053 | 447 | 0 | 705 | 382 |
| RTOR Reduction (vph) | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 143 | 0 | 0 | 226 |
| Lane Group Flow (vph) | 436 | 356 | 0 | 0 | 0 | 0 | 225 | 1053 | 304 | 0 | 705 | 156 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | | | 5 | | 2 | | | 6 | | |
| Permitted Phases | | | | | 2 | | | | 6 | | | |
| Actuated Green, G (s) | 40.1 | 40.1 | | | 37.0 | | 101.9 | 101.9 | | 60.9 | | 60.9 |
| Effective Green, g (s) | 40.1 | 40.1 | | | 37.0 | | 101.9 | 101.9 | | 60.9 | | 60.9 |
| Actuated g/C Ratio | 0.27 | 0.27 | | | 0.25 | | 0.68 | 0.68 | | 0.41 | | 0.41 |
| Clearance Time (s) | 4.0 | 4.0 | | | 4.0 | | 4.0 | 4.0 | | 4.0 | | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | 3.0 | 3.0 | | 3.0 | | 3.0 |
| Lane Grp Cap (vph) | 473 | 883 | | | 437 | | 2404 | 1075 | | 1437 | | 643 |
| v/s Ratio Prot | c0.25 | 0.14 | | | c0.13 | | 0.30 | 0.20 | | 0.20 | | |
| v/s Ratio Perm | | | | | 0.28 | | | | 0.28 | | 0.24 | |
| v/c Ratio | 0.92 | 0.40 | | | 0.51 | | 0.44 | 0.28 | | 0.49 | | 0.24 |
| Uniform Delay, d1 | 53.4 | 45.1 | | | 48.8 | | 11.0 | 9.5 | | 33.0 | | 29.3 |
| Progression Factor | 1.00 | 1.00 | | | 0.96 | | 0.83 | 0.32 | | 0.26 | | 0.05 |
| Incremental Delay, d2 | 23.5 | 0.3 | | | 0.9 | | 0.5 | 0.6 | | 0.9 | | 0.7 |
| Delay (s) | 76.9 | 45.4 | | | 47.8 | | 9.6 | 3.7 | | 9.3 | | 2.2 |
| Level of Service | E | D | | | D | | A | A | | A | | A |
| Approach Delay (s) | 60.6 | | 0.0 | | 13.0 | | 6.8 | | | | | |
| Approach LOS | E | | A | | B | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 22.8 | | HCM Level of Service | | C | | | | | | | |
| HCM Volume to Capacity ratio | 0.67 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 61.6% | | ICU Level of Service | | B | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|-------|------|------|-------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Satd. Flow (prot) | 1831 | 1583 | 1770 | 1583 | 1583 | 1583 | 3529 | 1770 | 1770 | 3539 | 1770 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.46 | 1.00 | 1.00 | 1.00 | 1.00 | 0.16 | 1.00 | 0.16 | 1.00 | 0.16 |
| Satd. Flow (perm) | 1831 | 1583 | 849 | 1583 | 1583 | 1583 | 3529 | 290 | 290 | 3539 | 290 | 3539 |
| Volume (vph) | 35 | 64 | 175 | 17 | 0 | 31 | 0 | 1354 | 27 | 121 | 611 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 38 | 70 | 190 | 18 | 0 | 34 | 0 | 1472 | 29 | 132 | 664 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 172 | 0 | 0 | 31 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 108 | 18 | 18 | 0 | 3 | 0 | 1500 | 0 | 132 | 664 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | 4 | 8 | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 13.9 | 13.9 | 13.9 | 13.9 | | | 128.1 | | | 128.1 | | |
| Effective Green, g (s) | 13.9 | 13.9 | 13.9 | 13.9 | | | 128.1 | | | 128.1 | | |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.09 | 0.09 | | | 0.85 | | | 0.85 | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | | | 4.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 170 | 147 | 79 | 147 | | | 3014 | | | 248 | | |
| v/s Ratio Prot | | | | 0.43 | | | 0.19 | | | | | |
| v/s Ratio Perm | 0.06 | 0.12 | 0.02 | 0.02 | | | c0.46 | | | | | |
| v/c Ratio | 0.64 | 0.12 | 0.23 | 0.02 | | | 0.50 | | | 0.22 | | |
| Uniform Delay, d1 | 65.6 | 62.4 | 63.1 | 61.9 | | | 2.8 | | | 2.9 | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | | | 1.00 | | | 1.66 | | |
| Incremental Delay, d2 | 7.5 | 0.4 | 1.5 | 0.1 | | | 0.6 | | | 7.2 | | |
| Delay (s) | 73.2 | 62.8 | 64.5 | 61.9 | | | 3.4 | | | 12.1 | | |
| Level of Service | E | E | E | E | | | A | | | B | | |
| Approach Delay (s) | 66.6 | | | 62.8 | | | 3.4 | | | 2.6 | | |
| Approach LOS | E | | | E | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 11.4 | | | HCM Level of Service | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.61 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 67.0% | | | ICU Level of Service | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Baseline + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|-------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 46 | 632 | 49 | 104 | 305 | 85 | 57 | 8 | 107 | 61 | 12 | 19 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 50 | 687 | 53 | 113 | 332 | 92 | 62 | 9 | 116 | 66 | 13 | 21 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | None | | | | | | None | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 424 | | | 740 | | | 1398 | | | 1464 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 424 | | | 740 | | | 1398 | | | 1464 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | |
| p0 queue free % | 96 | | | 87 | | | 32 | | | 92 | | |
| cM capacity (veh/h) | 1135 | | | 866 | | | 91 | | | 107 | | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 50 | 740 | 113 | 424 | 71 | 116 | 100 | | | | | |
| Volume Left | 50 | 0 | 113 | 0 | 62 | 0 | 66 | | | | | |
| Volume Right | 0 | 53 | 0 | 92 | 0 | 116 | 21 | | | | | |
| cSH | 1135 | 1700 | 866 | 1700 | 93 | 432 | 92 | | | | | |
| Volume to Capacity | 0.04 | 0.44 | 0.13 | 0.25 | 0.76 | 0.27 | 1.08 | | | | | |
| Queue Length (ft) | 3 | 0 | 11 | 0 | 99 | 27 | 166 | | | | | |
| Control Delay (s) | 8.3 | 0.0 | 9.8 | 0.0 | 118.0 | 16.4 | 202.5 | | | | | |
| Lane LOS | A | A | A | F | C | F | F | | | | | |
| Approach Delay (s) | 0.5 | | | 2.1 | | | 54.8 | | | 202.5 | | |
| Approach LOS | E | | | F | | | F | | | F | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 19.8 | | | | | | | | | | | |
| Intersection Capacity Utilization | 63.8% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|-------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 0.95 | |
| Frt | 0.86 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Flt Permitted | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | | 3536 | |
| Volume (vph) | 0 | 0 | 37 | 0 | 0 | 0 | 16 | 619 | 0 | 0 | 1583 | 10 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 40 | 0 | 0 | 0 | 17 | 673 | 0 | 0 | 1721 | 11 |
| RTOR Reduction (vph) | 0 | 39 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 1 | 0 | 0 | 0 | 0 | 17 | 673 | 0 | 0 | 1732 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Effective Green, g (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 91.6 | | 91.6 | |
| Actuated g/C Ratio | 0.03 | | 0.03 | | 0.90 | | 0.83 | | 0.83 | | 0.83 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 48 | | 50 | | 3175 | | 2945 | | 2945 | | 2945 | |
| v/s Ratio Prot | c0.02 | | c0.01 | | 0.19 | | c0.49 | | c0.49 | | c0.49 | |
| v/s Ratio Perm | 0.00 | | c0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.00 | |
| v/c Ratio | 0.03 | | 0.34 | | 0.21 | | 0.59 | | 0.59 | | 0.59 | |
| Uniform Delay, d1 | 51.8 | | 52.4 | | 0.7 | | 3.0 | | 3.0 | | 3.0 | |
| Progression Factor | 1.00 | | 0.79 | | 0.61 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.2 | | 3.9 | | 0.1 | | 0.9 | | 0.9 | | 0.9 | |
| Delay (s) | 52.0 | | 45.6 | | 0.6 | | 3.9 | | 3.9 | | 3.9 | |
| Level of Service | D | | D | | A | | A | | A | | A | |
| Approach Delay (s) | 52.0 | | 0.0 | | 1.7 | | 3.9 | | 3.9 | | 3.9 | |
| Approach LOS | D | | A | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 4.1 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.59 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | A | | A | | A | | A | |
| Intersection Capacity Utilization | 54.1% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 0.95 | |
| Frt | 0.90 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (prot) | 1659 | | 1770 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Flt Permitted | 0.96 | | 0.75 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (perm) | 1618 | | 1403 | | 1583 | | 3539 | | 1583 | | 1583 | |
| Volume (vph) | 2 | 0 | 5 | 18 | 0 | 10 | 0 | 696 | 13 | 8 | 1761 | 1 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 2 | 0 | 5 | 20 | 0 | 11 | 0 | 757 | 14 | 9 | 1914 | 1 |
| RTOR Reduction (vph) | 0 | 4 | 0 | 0 | 0 | 9 | 0 | 0 | 4 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 3 | 0 | 0 | 20 | 2 | 0 | 757 | 10 | 9 | 1914 | 1 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | 6 |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 85.2 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 85.2 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.73 | | 0.73 | | 0.77 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 247 | | 214 | | 242 | | 2574 | | 1151 | | 1226 | |
| v/s Ratio Prot | 0.00 | | c0.01 | | 0.01 | | 0.21 | | 0.01 | | c0.54 | |
| v/s Ratio Perm | 0.01 | | 0.09 | | 0.01 | | 0.29 | | 0.01 | | 0.70 | |
| v/c Ratio | 0.01 | | 0.09 | | 0.01 | | 0.29 | | 0.01 | | 0.70 | |
| Uniform Delay, d1 | 39.6 | | 40.1 | | 39.5 | | 5.2 | | 4.1 | | 6.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.25 | | 1.89 | | 0.69 | |
| Incremental Delay, d2 | 0.0 | | 0.2 | | 0.0 | | 0.3 | | 0.0 | | 1.3 | |
| Delay (s) | 39.6 | | 40.2 | | 39.5 | | 6.8 | | 7.8 | | 5.5 | |
| Level of Service | D | | D | | D | | A | | A | | F | |
| Approach Delay (s) | 39.6 | | 40.0 | | 6.8 | | 5.9 | | 5.9 | | 5.9 | |
| Approach LOS | D | | D | | A | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 6.6 | | HCM Level of Service | | A | | A | | A | | A | |
| HCM Volume to Capacity ratio | 0.60 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | B | | B | | B | | B | |
| Intersection Capacity Utilization | 58.7% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|-------|-------|----------------------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 24 | 242 | 102 | 698 | 1872 | 11 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 26 | 263 | 111 | 759 | 2035 | 12 |
| RTOR Reduction (vph) | 0 | 168 | 0 | 0 | 0 | 3 |
| Lane Group Flow (vph) | 26 | 95 | 111 | 759 | 2035 | 9 |
| Turn Type | Perm | Perm | Prot | | Perm | Perm |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | | 4 | | | | 6 |
| Actuated Green, G (s) | 11.5 | 11.5 | 11.4 | 90.5 | 75.1 | 75.1 |
| Effective Green, g (s) | 11.5 | 11.5 | 11.4 | 90.5 | 75.1 | 75.1 |
| Actuated g/C Ratio | 0.10 | 0.10 | 0.10 | 0.82 | 0.68 | 0.68 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 185 | 165 | 183 | 2912 | 2416 | 1081 |
| v/s Ratio Prot | 0.01 | | c0.06 | 0.21 | c0.57 | |
| v/s Ratio Perm | | 0.17 | | | | 0.01 |
| v/c Ratio | 0.14 | 0.57 | 0.61 | 0.26 | 0.84 | 0.01 |
| Uniform Delay, d1 | 44.8 | 46.9 | 47.2 | 2.2 | 13.0 | 5.6 |
| Progression Factor | 1.00 | 1.00 | 0.98 | 0.93 | 0.75 | 1.24 |
| Incremental Delay, d2 | 0.3 | 4.8 | 5.5 | 0.2 | 3.0 | 0.0 |
| Delay (s) | 45.1 | 51.7 | 51.8 | 2.3 | 12.7 | 6.9 |
| Level of Service | D | D | D | A | B | A |
| Approach Delay (s) | 51.1 | | | 8.6 | 12.7 | |
| Approach LOS | D | | | A | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | | 15.0 | | HCM Level of Service | | B |
| HCM Volume to Capacity ratio | | 0.90 | | | | |
| Actuated Cycle Length (s) | | 110.0 | | Sum of lost time (s) | 12.0 | |
| Intersection Capacity Utilization | | 73.4% | | ICU Level of Service | | D |
| Analysis Period (min) | | 15 | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|-------|------|----------------------|------|------|-------|-------|------|------|------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.93 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.98 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1782 | 1583 | 1696 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.73 | 1.00 | 0.85 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1353 | 1583 | 1475 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 28 | 3 | 148 | 25 | 3 | 29 | 102 | 698 | 27 | 11 | 2170 | 22 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 30 | 3 | 161 | 27 | 3 | 32 | 111 | 759 | 29 | 12 | 2359 | 24 |
| RTOR Reduction (vph) | 0 | 0 | 107 | 0 | 29 | 0 | 0 | 0 | 6 | 0 | 0 | 6 |
| Lane Group Flow (vph) | 0 | 33 | 54 | 0 | 33 | 0 | 111 | 759 | 23 | 12 | 2359 | 18 |
| Turn Type | Perm | Perm | Perm | Perm | Perm | Prot | Perm | Prot | Perm | Prot | Perm | Perm |
| Protected Phases | | 4 | | 8 | | 5 | 2 | | 2 | 1 | 6 | |
| Permitted Phases | 4 | | 4 | 8 | | | | | | | | 6 |
| Actuated Green, G (s) | 9.5 | 9.5 | 9.5 | 10.2 | 87.6 | 87.6 | 0.9 | 78.3 | 78.3 | | | |
| Effective Green, g (s) | 9.5 | 9.5 | 9.5 | 10.2 | 87.6 | 87.6 | 0.9 | 78.3 | 78.3 | | | |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.09 | 0.09 | 0.80 | 0.80 | 0.01 | 0.71 | 0.71 | | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 117 | 137 | 127 | 164 | 2818 | 1261 | 14 | 2519 | 1127 | | | |
| v/s Ratio Prot | | | | c0.06 | 0.21 | | 0.01 | c0.67 | | | | |
| v/s Ratio Perm | 0.02 | 0.10 | 0.04 | | | 0.02 | | | 0.02 | | | 0.02 |
| v/c Ratio | 0.28 | 0.39 | 0.26 | 0.68 | 0.27 | 0.02 | 0.86 | 0.94 | 0.02 | | | |
| Uniform Delay, d1 | 47.1 | 47.5 | 47.0 | 48.3 | 2.9 | 2.3 | 54.5 | 13.7 | 4.6 | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.28 | 0.26 | 0.04 | | | |
| Incremental Delay, d2 | 1.3 | 1.9 | 1.1 | 10.5 | 0.2 | 0.0 | 121.3 | 5.3 | 0.0 | | | |
| Delay (s) | 48.4 | 49.4 | 48.0 | 58.8 | 3.1 | 2.3 | 190.9 | 8.9 | 0.2 | | | |
| Level of Service | D | D | D | E | A | A | F | A | A | | | |
| Approach Delay (s) | 49.2 | | 48.0 | | 10.0 | | | 9.7 | | | | |
| Approach LOS | D | | D | | A | | | A | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | 12.6 | | HCM Level of Service | | | | B | | | | |
| HCM Volume to Capacity ratio | | 0.93 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 110.0 | | Sum of lost time (s) | 12.0 | | | | | | | |
| Intersection Capacity Utilization | | 85.6% | | ICU Level of Service | | | | E | | | | |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|-------|------|-------|-------|------|------|------|------|-------|------|------|
| Lane Configurations | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.96 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1786 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1786 | | 1681 | 1733 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 100 | 58 | 22 | 184 | 78 | 35 | 117 | 460 | 135 | 160 | 1641 | 788 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 109 | 63 | 24 | 200 | 85 | 38 | 127 | 500 | 147 | 174 | 1784 | 857 |
| RTOR Reduction (vph) | 0 | 14 | 0 | 0 | 0 | 33 | 0 | 0 | 75 | 0 | 0 | 288 |
| Lane Group Flow (vph) | 109 | 73 | 0 | 139 | 146 | 5 | 127 | 500 | 72 | 174 | 1784 | 569 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | | | 8 | | | 2 | | | 6 | |
| Actuated Green, G (s) | 9.7 | 9.7 | | 13.9 | 13.9 | 13.9 | 9.8 | 52.6 | 52.6 | 14.8 | 57.6 | 57.6 |
| Effective Green, g (s) | 9.7 | 9.7 | | 13.9 | 13.9 | 13.9 | 9.8 | 52.6 | 52.6 | 14.8 | 57.6 | 57.6 |
| Actuated g/C Ratio | 0.09 | 0.09 | | 0.13 | 0.13 | 0.13 | 0.09 | 0.49 | 0.49 | 0.14 | 0.54 | 0.54 |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 311 | 162 | | 218 | 225 | 206 | 162 | 1740 | 778 | 245 | 1905 | 852 |
| v/s Ratio Prot | 0.03 | c0.05 | | 0.08 | c0.08 | | 0.07 | 0.14 | | c0.10 | 0.50 | |
| v/s Ratio Perm | | | | | 0.02 | | | 0.09 | | | | 0.54 |
| v/c Ratio | 0.35 | 0.45 | | 0.64 | 0.65 | 0.02 | 0.78 | 0.29 | 0.09 | 0.71 | 0.94 | 0.67 |
| Uniform Delay, d1 | 45.7 | 46.1 | | 44.2 | 44.2 | 40.6 | 47.6 | 16.1 | 14.5 | 44.1 | 23.0 | 17.8 |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Incremental Delay, d2 | 0.7 | 2.0 | | 6.0 | 6.3 | 0.0 | 21.5 | 0.4 | 0.2 | 9.3 | 10.3 | 4.1 |
| Delay (s) | 46.4 | 48.1 | | 50.2 | 50.5 | 40.7 | 69.1 | 16.5 | 14.7 | 53.4 | 33.2 | 22.0 |
| Level of Service | D | D | | D | D | D | E | B | B | D | C | C |
| Approach Delay (s) | | 47.2 | | | 49.2 | | | 24.8 | | | 31.1 | |
| Approach LOS | | D | | | D | | | C | | | C | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | 32.1 | | | | | | | | | | C |
| HCM Volume to Capacity ratio | | 0.85 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 107.0 | | | | | | 12.0 | | | | |
| Intersection Capacity Utilization | | 75.7% | | | | | | | | | | D |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|-------|------|------|------|------|-------|------|------|------|------|------|
| Lane Configurations | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ | ↔ | ↑ | ↘ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3423 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3423 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 243 | 234 | 65 | 0 | 0 | 0 | 242 | 818 | 429 | 0 | 586 | 605 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 264 | 254 | 71 | 0 | 0 | 0 | 263 | 889 | 466 | 0 | 637 | 658 |
| RTOR Reduction (vph) | 0 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 79 | 0 | 0 | 370 |
| Lane Group Flow (vph) | 264 | 300 | 0 | 0 | 0 | 0 | 263 | 889 | 387 | 0 | 637 | 288 |
| Turn Type | Prot | | | | | Prot | Prot | | Perm | | | Perm |
| Protected Phases | 7 | 4 | | | | | 5 | 2 | | | 6 | |
| Permitted Phases | | | | | | | | 2 | | | | 6 |
| Actuated Green, G (s) | 20.9 | 20.9 | | | | | 29.0 | 81.1 | 81.1 | | 48.1 | 48.1 |
| Effective Green, g (s) | 20.9 | 20.9 | | | | | 29.0 | 81.1 | 81.1 | | 48.1 | 48.1 |
| Actuated g/C Ratio | 0.19 | 0.19 | | | | | 0.26 | 0.74 | 0.74 | | 0.44 | 0.44 |
| Clearance Time (s) | 4.0 | 4.0 | | | | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | | | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 336 | 650 | | | | | 467 | 2609 | 1167 | | 1548 | 692 |
| v/s Ratio Prot | c0.15 | 0.09 | | | | | c0.15 | 0.25 | | | 0.18 | |
| v/s Ratio Perm | | | | | | | | | 0.29 | | | 0.42 |
| v/c Ratio | 0.79 | 0.46 | | | | | 0.56 | 0.34 | 0.33 | | 0.41 | 0.42 |
| Uniform Delay, d1 | 42.4 | 39.6 | | | | | 35.0 | 5.1 | 5.0 | | 21.2 | 21.3 |
| Progression Factor | 1.00 | 1.00 | | | | | 0.98 | 0.72 | 0.43 | | 1.00 | 1.00 |
| Incremental Delay, d2 | 11.4 | 0.5 | | | | | 1.4 | 0.3 | 0.7 | | 0.8 | 1.8 |
| Delay (s) | 53.9 | 40.1 | | | | | 35.8 | 4.0 | 2.9 | | 22.0 | 23.1 |
| Level of Service | D | D | | | | | D | A | A | | C | C |
| Approach Delay (s) | | 46.3 | | | | | | 0.0 | 8.8 | | 22.6 | |
| Approach LOS | | D | | | | | | A | A | | C | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | 20.2 | | | | | | | | | | C |
| HCM Volume to Capacity ratio | | 0.80 | | | | | | | | | | |
| Actuated Cycle Length (s) | | 110.0 | | | | | | | | | 12.0 | |
| Intersection Capacity Utilization | | 57.5% | | | | | | | | | | B |
| Analysis Period (min) | | 15 | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|-------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 |
| Satd. Flow (prot) | 1833 | 1583 | 1770 | 1583 | 1583 | 3530 | 1770 | 3539 | 1770 | 3539 | 1770 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.72 | 1.00 | 1.00 | 1.00 | 1.00 | 0.18 | 1.00 | 0.18 | 1.00 | 0.18 |
| Satd. Flow (perm) | 1833 | 1583 | 1342 | 1583 | 1583 | 3530 | 343 | 3539 | 343 | 3539 | 343 | 3539 |
| Volume (vph) | 17 | 35 | 143 | 13 | 0 | 147 | 0 | 1213 | 22 | 87 | 532 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 18 | 38 | 155 | 14 | 0 | 160 | 0 | 1318 | 24 | 95 | 578 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 139 | 0 | 0 | 59 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 56 | 16 | 14 | 0 | 101 | 0 | 1341 | 0 | 95 | 578 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | 4 | 8 | 8 | 8 | 2 | 6 | 6 | 6 | 6 | 6 | 6 |
| Permitted Phases | 4 | 4 | 8 | 8 | 8 | 2 | 6 | 6 | 6 | 6 | 6 | 6 |
| Actuated Green, G (s) | 11.5 | 11.5 | 11.5 | 11.5 | 11.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 |
| Effective Green, g (s) | 11.5 | 11.5 | 11.5 | 11.5 | 11.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 | 90.5 |
| Actuated g/C Ratio | 0.10 | 0.10 | 0.10 | 0.10 | 0.10 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 192 | 165 | 140 | 165 | 165 | 2904 | 282 | 2912 | 282 | 2912 | 282 | 2912 |
| v/s Ratio Prot | | | | | | c0.38 | | 0.16 | | | | |
| v/s Ratio Perm | 0.03 | 0.10 | 0.01 | 0.10 | 0.10 | 0.28 | | 0.28 | | | | |
| v/c Ratio | 0.29 | 0.10 | 0.10 | 0.61 | 0.46 | 0.34 | 0.20 | | | | | |
| Uniform Delay, d1 | 45.5 | 44.6 | 44.6 | 47.1 | 2.8 | 2.4 | 2.1 | | | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 2.07 | 0.36 | | | | | |
| Incremental Delay, d2 | 0.8 | 0.3 | 0.3 | 6.6 | 0.5 | 3.0 | 0.1 | | | | | |
| Delay (s) | 46.3 | 44.8 | 44.9 | 53.7 | 3.3 | 7.9 | 0.9 | | | | | |
| Level of Service | D | D | D | D | A | A | A | | | | | |
| Approach Delay (s) | 45.2 | | 53.0 | | 3.3 | | 1.9 | | | | | |
| Approach LOS | D | | D | | A | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 10.2 | | | HCM Level of Service | | | | B | | | | |
| HCM Volume to Capacity ratio | 0.52 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | Sum of lost time (s) | | | | 8.0 | | | | |
| Intersection Capacity Utilization | 58.5% | | | ICU Level of Service | | | | B | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Cumulative AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 28 | 217 | 52 | 186 | 698 | 39 | 92 | 14 | 82 | 30 | 9 | 56 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 30 | 236 | 57 | 202 | 759 | 42 | 100 | 15 | 89 | 33 | 10 | 61 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | None | | | | | | None | | | | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 801 | 292 | | | 1554 | 1530 | 264 | 1489 | 1538 | 780 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | |
| vCu, unblocked vol | 801 | 292 | | | 1554 | 1530 | 264 | 1489 | 1538 | 780 | | |
| tC, single (s) | 4.1 | 4.1 | | | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | | |
| tC, 2 stage (s) | | | | | | | | | | | | |
| tF (s) | 2.2 | 2.2 | | | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | | |
| p0 queue free % | 96 | 84 | | | 0 | 84 | 88 | 52 | 90 | 85 | | |
| cM capacity (veh/h) | 822 | 1269 | | | 61 | 95 | 775 | 68 | 94 | 395 | | |
| Direction, Lane # | | | | | | | | | | | | |
| Volume Total | 30 | 292 | 202 | 801 | 115 | 89 | 103 | | | | | |
| Volume Left | 30 | 0 | 202 | 0 | 100 | 0 | 33 | | | | | |
| Volume Right | 0 | 57 | 0 | 42 | 0 | 89 | 61 | | | | | |
| cSH | 822 | 1700 | 1269 | 1700 | 64 | 775 | 140 | | | | | |
| Volume to Capacity | 0.04 | 0.17 | 0.16 | 0.47 | 1.79 | 0.12 | 0.74 | | | | | |
| Queue Length (ft) | 3 | 0 | 14 | 0 | 262 | 10 | 108 | | | | | |
| Control Delay (s) | 9.5 | 0.0 | 8.4 | 0.0 | 515.9 | 10.3 | 81.4 | | | | | |
| Lane LOS | A | A | A | F | B | F | | | | | | |
| Approach Delay (s) | 0.9 | 1.7 | | | 295.3 | 81.4 | | | | | | |
| Approach LOS | | F | | | F | F | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 43.3 | | | | | | | | | | | |
| Intersection Capacity Utilization | 64.9% | | | ICU Level of Service | | | | C | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|------|------|-------|------|------|------|
| Lane Configurations | ↔ | | | ↔ | | | | ↕ | ↕ | | ↕ | ↕ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | | | | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | | | | | 1.00 | | 0.95 | | 0.95 | |
| Frt | 0.90 | | | | | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 0.99 | | | | | | 0.95 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1651 | | | | | | 1770 | | 3539 | | 3528 | |
| Flt Permitted | 0.96 | | | | | | 0.95 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1602 | | | | | | 1770 | | 3539 | | 3528 | |
| Volume (vph) | 6 | 0 | 21 | 0 | 0 | 0 | 19 | 2257 | 0 | 0 | 861 | 18 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 7 | 0 | 23 | 0 | 0 | 0 | 21 | 2453 | 0 | 0 | 936 | 20 |
| RTOR Reduction (vph) | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 11 | 0 | 0 | 0 | 0 | 21 | 2453 | 0 | 0 | 955 | 0 |
| Turn Type | Prot | | Prot | | Prot | | | | | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | | | 6 | |
| Permitted Phases | | | | | | | | | | | | |
| Actuated Green, G (s) | 18.0 | | | | | | 2.8 | | 84.0 | | 77.2 | |
| Effective Green, g (s) | 18.0 | | | | | | 2.8 | | 84.0 | | 77.2 | |
| Actuated g/C Ratio | 0.16 | | | | | | 0.03 | | 0.76 | | 0.70 | |
| Clearance Time (s) | 4.0 | | | | | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | | | | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 262 | | | | | | 45 | | 2703 | | 2476 | |
| v/s Ratio Prot | | | | | | | 0.01 | | c0.69 | | 0.27 | |
| v/s Ratio Perm | c0.02 | | | | | | | | | | | |
| v/c Ratio | 0.04 | | | | | | 0.47 | | 0.91 | | 0.39 | |
| Uniform Delay, d1 | 38.7 | | | | | | 52.9 | | 10.0 | | 6.7 | |
| Progression Factor | 1.00 | | | | | | 0.89 | | 1.05 | | 1.00 | |
| Incremental Delay, d2 | 0.1 | | | | | | 3.5 | | 2.8 | | 0.5 | |
| Delay (s) | 38.8 | | | | | | 50.4 | | 13.4 | | 7.2 | |
| Level of Service | D | | | | | | D | | B | | A | |
| Approach Delay (s) | 38.8 | | 0.0 | | 13.7 | | 7.2 | | | | | |
| Approach LOS | D | | A | | B | | A | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 12.1 | | HCM Level of Service | | B | | | | | | | |
| HCM Volume to Capacity ratio | 0.77 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | | | | | | | |
| Intersection Capacity Utilization | 72.4% | | ICU Level of Service | | C | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | | | ↔ | | | | ↕ | ↕ | | ↕ | ↕ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | | | | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | | | | | 1.00 | | 0.95 | | 1.00 | |
| Frt | 0.97 | | | | | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.96 | | | | | | 0.95 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1735 | | | | | | 1770 | | 1583 | | 1583 | |
| Flt Permitted | 0.87 | | | | | | 0.75 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1571 | | | | | | 1397 | | 1583 | | 1583 | |
| Volume (vph) | 8 | 0 | 3 | 13 | 0 | 17 | 3 | 2205 | 23 | 12 | 839 | 4 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 0 | 3 | 14 | 0 | 18 | 3 | 2397 | 25 | 13 | 912 | 4 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 15 | 0 | 0 | 5 | 0 | 0 | 1 |
| Lane Group Flow (vph) | 0 | 9 | 0 | 0 | 14 | 3 | 3 | 2397 | 20 | 13 | 912 | 3 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | | | | | 8 | | 2 | | 6 | |
| Actuated Green, G (s) | 16.8 | | | | 16.8 | | 16.8 | | 1.2 | | 78.8 | |
| Effective Green, g (s) | 16.8 | | | | 16.8 | | 16.8 | | 1.2 | | 78.8 | |
| Actuated g/C Ratio | 0.15 | | | | 0.15 | | 0.15 | | 0.01 | | 0.72 | |
| Clearance Time (s) | 4.0 | | | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 240 | | | | 213 | | 242 | | 19 | | 2535 | |
| v/s Ratio Prot | | | | | | | 0.00 | | c0.68 | | c0.01 | |
| v/s Ratio Perm | 0.01 | | | | 0.01 | | 0.01 | | 0.02 | | 0.00 | |
| v/c Ratio | 0.04 | | | | 0.07 | | 0.01 | | 0.16 | | 0.95 | |
| Uniform Delay, d1 | 39.7 | | | | 39.9 | | 39.6 | | 53.9 | | 13.7 | |
| Progression Factor | 1.00 | | | | 1.00 | | 1.00 | | 1.03 | | 0.83 | |
| Incremental Delay, d2 | 0.1 | | | | 0.1 | | 0.0 | | 2.3 | | 5.9 | |
| Delay (s) | 39.8 | | | | 40.0 | | 39.6 | | 57.6 | | 17.3 | |
| Level of Service | D | | | | D | | D | | E | | B | |
| Approach Delay (s) | 39.8 | | 39.8 | | 17.2 | | 4.7 | | 4.7 | | 4.7 | |
| Approach LOS | D | | D | | B | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 14.1 | | HCM Level of Service | | B | | | | | | | |
| HCM Volume to Capacity ratio | 0.78 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 77.6% | | ICU Level of Service | | D | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↕ | ↕ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 22 | 111 | 197 | 2216 | 854 | 28 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 24 | 121 | 214 | 2409 | 928 | 30 |
| RTOR Reduction (vph) | 0 | 113 | 0 | 0 | 0 | 11 |
| Lane Group Flow (vph) | 24 | 8 | 214 | 2409 | 928 | 19 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | | 2 | |
| Permitted Phases | 4 | | 6 | | 6 | |
| Actuated Green, G (s) | 7.4 | 7.4 | 22.0 | 94.6 | 68.6 | 68.6 |
| Effective Green, g (s) | 7.4 | 7.4 | 22.0 | 94.6 | 68.6 | 68.6 |
| Actuated g/C Ratio | 0.07 | 0.07 | 0.20 | 0.86 | 0.62 | 0.62 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 119 | 106 | 354 | 3044 | 2207 | 987 |
| v/s Ratio Prot | 0.01 | | 0.12 | | c0.68 | |
| v/s Ratio Perm | 0.08 | | 0.26 | | 0.02 | |
| v/c Ratio | 0.20 | 0.08 | 0.60 | 0.79 | 0.42 | 0.02 |
| Uniform Delay, d1 | 48.5 | 48.1 | 40.0 | 3.4 | 10.6 | 7.9 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.33 | 2.93 |
| Incremental Delay, d2 | 0.8 | 0.3 | 2.9 | 2.2 | 0.6 | 0.0 |
| Delay (s) | 49.3 | 48.4 | 42.9 | 5.6 | 14.7 | 23.1 |
| Level of Service | D | D | D | A | B | C |
| Approach Delay (s) | 48.6 | | 8.6 | | 14.9 | |
| Approach LOS | D | | A | | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | 11.8 | | HCM Level of Service | | B | |
| HCM Volume to Capacity ratio | 0.82 | | Sum of lost time (s) | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | |
| Intersection Capacity Utilization | 71.3% | | Analysis Period (min) | | 15 | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|-----------------------|------|------|------|------|-------|------|-------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.97 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.96 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1781 | 1583 | 1741 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.79 | 1.00 | 0.75 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1469 | 1583 | 1366 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 21 | 2 | 94 | 24 | 0 | 6 | 133 | 2399 | 36 | 15 | 1006 | 26 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 23 | 2 | 102 | 26 | 0 | 7 | 145 | 2608 | 39 | 16 | 1093 | 28 |
| RTOR Reduction (vph) | 0 | 0 | 96 | 0 | 7 | 0 | 0 | 0 | 5 | 0 | 0 | 7 |
| Lane Group Flow (vph) | 0 | 25 | 6 | 0 | 26 | 0 | 145 | 2608 | 34 | 16 | 1093 | 21 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 8 | | 8 | | 5 | | 2 | | 6 | |
| Actuated Green, G (s) | 8.2 | | 8.2 | | 8.2 | | 25.4 | | 126.5 | | 104.4 | |
| Effective Green, g (s) | 8.2 | | 8.2 | | 8.2 | | 25.4 | | 126.5 | | 104.4 | |
| Actuated g/C Ratio | 0.05 | | 0.05 | | 0.05 | | 0.17 | | 0.84 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 80 | | 87 | | 75 | | 300 | | 2985 | | 1102 | |
| v/s Ratio Prot | 0.02 | | 0.06 | | 0.02 | | 0.08 | | c0.74 | | c0.31 | |
| v/s Ratio Perm | 0.31 | | 0.06 | | 0.35 | | 0.48 | | 0.87 | | 0.02 | |
| v/c Ratio | 0.31 | | 0.06 | | 0.35 | | 0.48 | | 0.87 | | 0.02 | |
| Uniform Delay, d1 | 68.2 | | 67.3 | | 68.3 | | 56.4 | | 7.0 | | 10.0 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 0.77 | | 0.14 | | 0.00 | |
| Incremental Delay, d2 | 2.2 | | 0.3 | | 2.8 | | 0.1 | | 0.4 | | 0.0 | |
| Delay (s) | 70.4 | | 67.6 | | 71.2 | | 43.5 | | 1.4 | | 10.6 | |
| Level of Service | E | | E | | E | | D | | A | | A | |
| Approach Delay (s) | 68.1 | | 71.2 | | 71.2 | | 3.5 | | 3.5 | | 11.5 | |
| Approach LOS | E | | E | | E | | A | | A | | B | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.3 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.89 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | ICU Level of Service | | | E | | | | | |
| Intersection Capacity Utilization | 88.0% | | | Analysis Period (min) | | | 15 | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|------|-------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.90 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 3433 | 1685 | | 1681 | 1746 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 543 | 69 | 121 | 111 | 66 | 121 | 395 | 2018 | 78 | 93 | 862 | 102 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 590 | 75 | 132 | 121 | 72 | 132 | 429 | 2193 | 85 | 101 | 937 | 111 |
| RTOR Reduction (vph) | 0 | 43 | 0 | 0 | 0 | 91 | 0 | 0 | 19 | 0 | 0 | 58 |
| Lane Group Flow (vph) | 590 | 164 | 0 | 94 | 99 | 41 | 429 | 2193 | 66 | 101 | 937 | 53 |
| Turn Type | Split | | Split | | Perm | | Prot | | Prot | | Perm | |
| Protected Phases | 4 | 4 | 8 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | | | 8 | | 8 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 22.0 | 22.0 | 13.1 | 13.1 | 13.1 | 47.0 | 87.7 | 87.7 | 11.2 | 51.9 | 51.9 | |
| Effective Green, g (s) | 22.0 | 22.0 | 13.1 | 13.1 | 13.1 | 47.0 | 87.7 | 87.7 | 11.2 | 51.9 | 51.9 | |
| Actuated g/C Ratio | 0.15 | 0.15 | 0.09 | 0.09 | 0.09 | 0.31 | 0.58 | 0.58 | 0.07 | 0.35 | 0.35 | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 504 | 247 | 147 | 152 | 138 | 555 | 2069 | 926 | 132 | 1224 | 548 | |
| v/s Ratio Prot | c0.17 | 0.12 | 0.06 | 0.06 | | 0.24 | c0.62 | | 0.06 | c0.26 | | |
| v/s Ratio Perm | | | | | 0.08 | | | 0.05 | | | | 0.07 |
| v/c Ratio | 1.17 | 0.67 | 0.64 | 0.65 | 0.30 | 0.77 | 1.06 | 0.07 | 0.77 | 0.77 | 0.10 | |
| Uniform Delay, d1 | 64.0 | 60.5 | 66.2 | 66.2 | 64.1 | 46.7 | 31.1 | 13.5 | 68.1 | 43.6 | 33.2 | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.96 | 1.38 | 0.87 | 0.83 | 1.18 | |
| Incremental Delay, d2 | 96.3 | 6.6 | 8.8 | 9.6 | 1.2 | 6.3 | 37.5 | 0.1 | 21.3 | 4.3 | 0.3 | |
| Delay (s) | 160.3 | 67.1 | 75.0 | 75.8 | 65.3 | 53.0 | 67.4 | 18.8 | 80.7 | 40.4 | 39.5 | |
| Level of Service | F | E | E | E | E | D | E | B | F | D | D | |
| Approach Delay (s) | 136.1 | | 71.3 | | 63.5 | | 43.9 | | | | | |
| Approach LOS | F | | E | | E | | D | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 71.1 | | HCM Level of Service | | E | | | | | | | |
| HCM Volume to Capacity ratio | 1.05 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 16.0 | | | | | | | |
| Intersection Capacity Utilization | 94.6% | | ICU Level of Service | | F | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|-------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.93 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 446 | 261 | 208 | 0 | 0 | 0 | 224 | 1088 | 445 | 0 | 719 | 390 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 485 | 284 | 226 | 0 | 0 | 0 | 243 | 1183 | 484 | 0 | 782 | 424 |
| RTOR Reduction (vph) | 0 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 148 | 0 | 0 | 238 |
| Lane Group Flow (vph) | 485 | 399 | 0 | 0 | 0 | 0 | 243 | 1183 | 336 | 0 | 782 | 186 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | | | 5 | | 2 | | | 6 | | |
| Permitted Phases | | | 2 | | 2 | | 2 | | 2 | | 6 | |
| Actuated Green, G (s) | 44.6 | 44.6 | | | 37.0 | | 97.4 | 97.4 | 56.4 | | 56.4 | |
| Effective Green, g (s) | 44.6 | 44.6 | | | 37.0 | | 97.4 | 97.4 | 56.4 | | 56.4 | |
| Actuated g/C Ratio | 0.30 | 0.30 | | | 0.25 | | 0.65 | 0.65 | 0.38 | | 0.38 | |
| Clearance Time (s) | 4.0 | 4.0 | | | 4.0 | | 4.0 | 4.0 | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | 3.0 | | 3.0 | 3.0 | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 526 | 982 | | | 437 | | 2298 | 1028 | 1331 | | 595 | |
| v/s Ratio Prot | c0.27 | 0.15 | | | c0.14 | | 0.33 | | 0.22 | | | |
| v/s Ratio Perm | | | | | | | | 0.31 | | | 0.27 | |
| v/c Ratio | 0.92 | 0.41 | | | 0.56 | | 0.51 | 0.33 | 0.59 | | 0.31 | |
| Uniform Delay, d1 | 51.0 | 42.1 | | | 49.3 | | 13.9 | 11.7 | 37.5 | | 33.1 | |
| Progression Factor | 1.00 | 1.00 | | | 0.95 | | 0.83 | 0.36 | 0.34 | | 0.13 | |
| Incremental Delay, d2 | 21.8 | 0.3 | | | 1.4 | | 0.7 | 0.7 | 1.3 | | 0.9 | |
| Delay (s) | 72.8 | 42.4 | | | 48.3 | | 12.2 | 5.0 | 13.9 | | 5.2 | |
| Level of Service | E | D | | | D | | B | A | B | | A | |
| Approach Delay (s) | 57.2 | | 0.0 | | 15.0 | | 10.8 | | | | | |
| Approach LOS | E | | A | | B | | B | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 24.0 | | HCM Level of Service | | C | | | | | | | |
| HCM Volume to Capacity ratio | 0.74 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 67.0% | | ICU Level of Service | | C | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Road

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|-------|-------|-------|-------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1830 | 1583 | 1770 | 1583 | 1583 | 1583 | 3529 | 1770 | 3539 | 1770 | 3539 | 1770 |
| Flt Permitted | 0.98 | 1.00 | 0.43 | 1.00 | 1.00 | 1.00 | 1.00 | 0.12 | 1.00 | 0.12 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1830 | 1583 | 804 | 1583 | 1583 | 1583 | 3529 | 232 | 3539 | 232 | 3539 | 232 |
| Volume (vph) | 38 | 69 | 189 | 18 | 0 | 34 | 0 | 1513 | 29 | 131 | 678 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 41 | 75 | 205 | 20 | 0 | 37 | 0 | 1645 | 32 | 142 | 737 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 185 | 0 | 0 | 33 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 116 | 20 | 20 | 0 | 4 | 0 | 1676 | 0 | 142 | 737 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | 4 | 8 | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 14.5 | 14.5 | 14.5 | 14.5 | | | 127.5 | 127.5 | 127.5 | 127.5 | | |
| Effective Green, g (s) | 14.5 | 14.5 | 14.5 | 14.5 | | | 127.5 | 127.5 | 127.5 | 127.5 | | |
| Actuated g/C Ratio | 0.10 | 0.10 | 0.10 | 0.10 | | | 0.85 | 0.85 | 0.85 | 0.85 | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | | | 4.0 | 4.0 | 4.0 | 4.0 | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | 3.0 | 3.0 | 3.0 | | |
| Lane Grp Cap (vph) | 177 | 153 | 78 | 153 | | | 3000 | 197 | 3008 | 3008 | | |
| v/s Ratio Prot | | | | 0.48 | | | 0.21 | | | | | |
| v/s Ratio Perm | 0.06 | 0.13 | 0.02 | 0.02 | | | c0.61 | | | | | |
| v/c Ratio | 0.66 | 0.13 | 0.26 | 0.02 | | | 0.56 | 0.72 | 0.25 | | | |
| Uniform Delay, d1 | 65.3 | 62.0 | 62.8 | 61.3 | | | 3.2 | 4.4 | 2.1 | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | | | 1.00 | 1.99 | 0.27 | | | |
| Incremental Delay, d2 | 8.4 | 0.4 | 1.7 | 0.1 | | | 0.8 | 17.6 | 0.2 | | | |
| Delay (s) | 73.8 | 62.4 | 64.5 | 61.4 | | | 4.0 | 26.3 | 0.7 | | | |
| Level of Service | E | E | E | E | | | A | C | A | | | |
| Approach Delay (s) | 66.5 | | | 62.5 | | | 4.0 | | | 4.9 | | |
| Approach LOS | E | | | E | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 12.2 | | | HCM Level of Service | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.78 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 72.4% | | | ICU Level of Service | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Avenue

Cumulative PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | | | | | | | | | | | | |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|-------|------|------|-----|--|--|------|--|--|------|--|--|-----|--|--|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | | | | | | | | | | | | |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | | | | | | | | | | | | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | | | | | | | | | | | | | |
| Volume (veh/h) | 50 | 662 | 53 | 113 | 316 | 92 | 62 | 9 | 116 | 66 | 14 | 21 | | | | | | | | | | | | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | | | | | | | | | | | | |
| Hourly flow rate (vph) | 54 | 720 | 58 | 123 | 343 | 100 | 67 | 10 | 126 | 72 | 15 | 23 | | | | | | | | | | | | |
| Pedestrians | | | | | | | | | | | | | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | | | | | | | | | | | | | |
| Median storage (veh) | | | | | | | | | | | | | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | | | | | | | | | | | | | |
| vC, conflicting volume | 443 | | | 777 | | | 1477 | | | 1546 | | | 748 | | | 1472 | | | 1525 | | | 393 | | |
| vC1, stage 1 conf vol | | | | | | | | | | | | | | | | | | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | | | | | | | | | | | | | | | | | | |
| vCu, unblocked vol | 443 | | | 777 | | | 1477 | | | 1546 | | | 748 | | | 1472 | | | 1525 | | | 393 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | | 6.2 | | | 7.1 | | | 6.5 | | | 6.2 | | |
| tC, 2 stage (s) | | | | | | | | | | | | | | | | | | | | | | | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | | 3.3 | | | 3.5 | | | 4.0 | | | 3.3 | | |
| p0 queue free % | 95 | | | 85 | | | 11 | | | 89 | | | 69 | | | 0 | | | 84 | | | 97 | | |
| cM capacity (veh/h) | 1117 | | | 839 | | | 76 | | | 93 | | | 412 | | | 57 | | | 96 | | | 655 | | |
| Direction, Lane # | | | | | | | | | | | | | | | | | | | | | | | | |
| Volume Total | 54 | 777 | 123 | 443 | 77 | 126 | 110 | | | | | | | | | | | | | | | | | |
| Volume Left | 54 | 0 | 123 | 0 | 67 | 0 | 72 | | | | | | | | | | | | | | | | | |
| Volume Right | 0 | 58 | 0 | 100 | 0 | 126 | 23 | | | | | | | | | | | | | | | | | |
| cSH | 1117 | 1700 | 839 | 1700 | 78 | 412 | 76 | | | | | | | | | | | | | | | | | |
| Volume to Capacity | 0.05 | 0.46 | 0.15 | 0.26 | 0.99 | 0.31 | 1.44 | | | | | | | | | | | | | | | | | |
| Queue Length (ft) | 4 | 0 | 13 | 0 | 134 | 32 | 221 | | | | | | | | | | | | | | | | | |
| Control Delay (s) | 8.4 | 0.0 | 10.0 | 0.0 | 193.8 | 17.5 | 353.4 | | | | | | | | | | | | | | | | | |
| Lane LOS | A | | B | | F | | C | | F | | F | | | | | | | | | | | | | |
| Approach Delay (s) | 0.5 | | | 2.2 | | | 84.5 | | | 353.4 | | | | | | | | | | | | | | |
| Approach LOS | A | | | B | | | F | | | F | | | | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | | | | | | | | | | | | | |
| Average Delay | 33.7 | | | | | | | | | | | | | | | | | | | | | | | |
| Intersection Capacity Utilization | 66.7% | | | ICU Level of Service | | | C | | | | | | | | | | | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|----------------------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 0.95 | | 1.00 | |
| Flt Protected | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1611 | | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | |
| Flt Permitted | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1611 | | 1611 | | 1770 | | 3539 | | 3536 | | 3536 | |
| Volume (vph) | 0 | 0 | 37 | 0 | 0 | 0 | 16 | 633 | 0 | 0 | 1589 | 10 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 40 | 0 | 0 | 0 | 17 | 688 | 0 | 0 | 1727 | 11 |
| RTOR Reduction (vph) | 0 | 39 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 1 | 0 | 0 | 0 | 0 | 17 | 688 | 0 | 0 | 1738 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 3.1 | | 91.6 | |
| Effective Green, g (s) | 3.3 | | 3.1 | | 98.7 | | 91.6 | | 3.1 | | 91.6 | |
| Actuated g/C Ratio | 0.03 | | 0.03 | | 0.90 | | 0.83 | | 0.03 | | 0.83 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 48 | | 50 | | 3175 | | 2945 | | 48 | | 2945 | |
| v/s Ratio Prot | c0.02 | | c0.01 | | 0.19 | | c0.49 | | c0.02 | | c0.49 | |
| v/s Ratio Perm | 0.03 | | 0.34 | | 0.22 | | 0.59 | | 0.03 | | 0.59 | |
| Uniform Delay, d1 | 51.8 | | 52.4 | | 0.7 | | 3.0 | | 51.8 | | 3.0 | |
| Progression Factor | 1.00 | | 0.79 | | 0.60 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.2 | | 3.9 | | 0.2 | | 0.9 | | 0.2 | | 0.9 | |
| Delay (s) | 52.0 | | 45.5 | | 0.6 | | 3.9 | | 52.0 | | 3.9 | |
| Level of Service | D | | D | | A | | A | | D | | A | |
| Approach Delay (s) | 52.0 | | 0.0 | | 1.7 | | 3.9 | | 52.0 | | 3.9 | |
| Approach LOS | D | | A | | A | | A | | D | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 4.0 | | HCM Level of Service | | A | | 4.0 | | HCM Level of Service | | A | |
| HCM Volume to Capacity ratio | 0.59 | | 0.59 | | 0.59 | | 0.59 | | 0.59 | | 0.59 | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 12.0 | | 110.0 | | Sum of lost time (s) | | 12.0 | |
| Intersection Capacity Utilization | 54.2% | | ICU Level of Service | | A | | 54.2% | | ICU Level of Service | | A | |
| Analysis Period (min) | 15 | | 15 | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|----------------------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (prot) | 1659 | | 1770 | | 1583 | | 3539 | | 1583 | | 1770 | |
| Flt Permitted | 0.96 | | 0.75 | | 1.00 | | 1.00 | | 0.95 | | 1.00 | |
| Satd. Flow (perm) | 1618 | | 1403 | | 1583 | | 3539 | | 1583 | | 1770 | |
| Volume (vph) | 2 | 0 | 5 | 18 | 0 | 10 | 0 | 710 | 13 | 8 | 1767 | 1 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 2 | 0 | 5 | 20 | 0 | 11 | 0 | 772 | 14 | 9 | 1921 | 1 |
| RTOR Reduction (vph) | 0 | 4 | 0 | 0 | 0 | 9 | 0 | 0 | 4 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 3 | 0 | 0 | 20 | 2 | 0 | 772 | 10 | 9 | 1921 | 1 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 80.0 | | 80.0 | | 1.2 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.73 | | 0.73 | | 0.01 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 247 | | 214 | | 242 | | 2574 | | 1151 | | 19 | |
| v/s Ratio Prot | c0.02 | | c0.01 | | 0.01 | | 0.22 | | 0.01 | | c0.54 | |
| v/s Ratio Perm | 0.00 | | c0.01 | | 0.01 | | 0.01 | | 0.01 | | 0.00 | |
| v/c Ratio | 0.01 | | 0.09 | | 0.01 | | 0.30 | | 0.01 | | 0.47 | |
| Uniform Delay, d1 | 39.6 | | 40.1 | | 39.5 | | 5.2 | | 4.1 | | 54.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.25 | | 1.89 | | 1.21 | |
| Incremental Delay, d2 | 0.0 | | 0.2 | | 0.0 | | 0.3 | | 0.0 | | 15.3 | |
| Delay (s) | 39.6 | | 40.2 | | 39.5 | | 6.8 | | 7.8 | | 80.7 | |
| Level of Service | D | | D | | D | | A | | A | | F | |
| Approach Delay (s) | 39.6 | | 40.0 | | 6.9 | | 5.9 | | 39.6 | | 6.9 | |
| Approach LOS | D | | D | | A | | A | | D | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 6.6 | | HCM Level of Service | | A | | 6.6 | | HCM Level of Service | | A | |
| HCM Volume to Capacity ratio | 0.60 | | 0.60 | | 0.60 | | 0.60 | | 0.60 | | 0.60 | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | | 110.0 | | Sum of lost time (s) | | 8.0 | |
| Intersection Capacity Utilization | 58.8% | | ICU Level of Service | | B | | 58.8% | | ICU Level of Service | | B | |
| Analysis Period (min) | 15 | | 15 | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|------|------|-------|------|----------------------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 24 | 242 | 102 | 712 | 1878 | 11 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 26 | 263 | 111 | 774 | 2041 | 12 |
| RTOR Reduction (vph) | 0 | 168 | 0 | 0 | 0 | 3 |
| Lane Group Flow (vph) | 26 | 95 | 111 | 774 | 2041 | 9 |
| Turn Type | Perm | Perm | Prot | | Perm | |
| Protected Phases | 4 | | 5 | 2 | 6 | |
| Permitted Phases | | 4 | | | | 6 |
| Actuated Green, G (s) | 11.5 | 11.5 | 11.4 | 90.5 | 75.1 | 75.1 |
| Effective Green, g (s) | 11.5 | 11.5 | 11.4 | 90.5 | 75.1 | 75.1 |
| Actuated g/C Ratio | 0.10 | 0.10 | 0.10 | 0.82 | 0.68 | 0.68 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 185 | 165 | 183 | 2912 | 2416 | 1081 |
| v/s Ratio Prot | 0.01 | | c0.06 | 0.22 | c0.58 | |
| v/s Ratio Perm | | 0.17 | | | | 0.01 |
| v/c Ratio | 0.14 | 0.57 | 0.61 | 0.27 | 0.84 | 0.01 |
| Uniform Delay, d1 | 44.8 | 46.9 | 47.2 | 2.2 | 13.1 | 5.6 |
| Progression Factor | 1.00 | 1.00 | 0.98 | 0.93 | 0.75 | 1.26 |
| Incremental Delay, d2 | 0.3 | 4.8 | 5.5 | 0.2 | 3.0 | 0.0 |
| Delay (s) | 45.1 | 51.7 | 51.8 | 2.3 | 12.8 | 7.0 |
| Level of Service | D | D | D | A | B | A |
| Approach Delay (s) | 51.1 | | | 8.5 | 12.8 | |
| Approach LOS | D | | | A | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | | | 15.0 | | HCM Level of Service | B |
| HCM Volume to Capacity ratio | | | 0.90 | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 12.0 |
| Intersection Capacity Utilization | | | 73.6% | | ICU Level of Service | D |
| Analysis Period (min) | | | 15 | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|------|------|-------|-------|----------------------|------|-------|-------|------|------|------|------|
| Lane Configurations | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.93 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.98 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1782 | 1583 | 1696 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.73 | 1.00 | 0.85 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1353 | 1583 | 1475 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 28 | 3 | 148 | 25 | 3 | 29 | 102 | 712 | 27 | 11 | 2176 | 22 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 30 | 3 | 161 | 27 | 3 | 32 | 111 | 774 | 29 | 12 | 2365 | 24 |
| RTOR Reduction (vph) | 0 | 0 | 107 | 0 | 29 | 0 | 0 | 0 | 6 | 0 | 0 | 6 |
| Lane Group Flow (vph) | 0 | 33 | 54 | 0 | 33 | 0 | 111 | 774 | 23 | 12 | 2365 | 18 |
| Turn Type | Perm | Perm | Perm | Perm | Perm | Prot | Perm | Prot | Perm | Prot | Perm | Perm |
| Protected Phases | | 4 | | 8 | | 5 | 2 | | 2 | 1 | 6 | |
| Permitted Phases | 4 | | 4 | | 8 | | | | | | | 6 |
| Actuated Green, G (s) | 9.5 | 9.5 | 9.5 | 10.2 | 87.6 | 87.6 | 0.9 | 78.3 | 78.3 | | | |
| Effective Green, g (s) | 9.5 | 9.5 | 9.5 | 10.2 | 87.6 | 87.6 | 0.9 | 78.3 | 78.3 | | | |
| Actuated g/C Ratio | 0.09 | 0.09 | 0.09 | 0.09 | 0.80 | 0.80 | 0.01 | 0.71 | 0.71 | | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 117 | 137 | 127 | 164 | 2818 | 1261 | 14 | 2519 | 1127 | | | |
| v/s Ratio Prot | | | | c0.06 | 0.22 | | 0.01 | c0.67 | | | | |
| v/s Ratio Perm | 0.02 | 0.10 | 0.04 | | | 0.02 | | | 0.02 | | | 0.02 |
| v/c Ratio | 0.28 | 0.39 | 0.26 | 0.68 | 0.27 | 0.02 | 0.86 | 0.94 | 0.02 | | | |
| Uniform Delay, d1 | 47.1 | 47.5 | 47.0 | 48.3 | 2.9 | 2.3 | 54.5 | 13.8 | 4.6 | | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.29 | 0.26 | 0.04 | | | |
| Incremental Delay, d2 | 1.3 | 1.9 | 1.1 | 10.5 | 0.2 | 0.0 | 120.9 | 5.4 | 0.0 | | | |
| Delay (s) | 48.4 | 49.4 | 48.0 | 58.8 | 3.2 | 2.3 | 191.0 | 9.0 | 0.2 | | | |
| Level of Service | D | D | D | E | A | A | F | A | A | | | |
| Approach Delay (s) | 49.2 | | 48.0 | | 9.9 | | | 9.8 | | | | |
| Approach LOS | D | | D | | A | | | A | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | | | 12.6 | | HCM Level of Service | B | | | | | | |
| HCM Volume to Capacity ratio | | | 0.93 | | | | | | | | | |
| Actuated Cycle Length (s) | | | 110.0 | | Sum of lost time (s) | 12.0 | | | | | | |
| Intersection Capacity Utilization | | | 85.8% | | ICU Level of Service | E | | | | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|-------|----------------------|-------|-------|------|-------|------|------|------|-------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.91 | |
| Frt | 1.00 | 0.94 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.95 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 3433 | 1752 | | 1681 | 1734 | 1583 | 1770 | 3539 | 1583 | 1770 | 4837 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 3433 | 1752 | | 1681 | 1734 | 1583 | 1770 | 3539 | 1583 | 1770 | 4837 | |
| Volume (vph) | 115 | 67 | 44 | 186 | 80 | 35 | 117 | 460 | 135 | 160 | 1643 | 793 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 125 | 73 | 48 | 202 | 87 | 38 | 127 | 500 | 147 | 174 | 1786 | 862 |
| RTOR Reduction (vph) | 0 | 22 | 0 | 0 | 0 | 33 | 0 | 0 | 79 | 0 | 69 | 0 |
| Lane Group Flow (vph) | 125 | 99 | 0 | 141 | 148 | 5 | 127 | 500 | 68 | 174 | 2579 | 0 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | | | 8 | | | 2 | | | | |
| Actuated Green, G (s) | 11.3 | 11.3 | | 13.7 | 13.7 | 13.7 | 9.8 | 51.2 | 51.2 | 17.8 | 59.2 | |
| Effective Green, g (s) | 11.3 | 11.3 | | 13.7 | 13.7 | 13.7 | 9.8 | 51.2 | 51.2 | 17.8 | 59.2 | |
| Actuated g/C Ratio | 0.10 | 0.10 | | 0.12 | 0.12 | 0.12 | 0.09 | 0.47 | 0.47 | 0.16 | 0.54 | |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 353 | 180 | | 209 | 216 | 197 | 158 | 1647 | 737 | 286 | 2603 | |
| v/s Ratio Prot | 0.04 | c0.07 | | 0.08 | c0.09 | | c0.07 | 0.14 | | 0.10 | c0.55 | |
| v/s Ratio Perm | | | | | 0.02 | | | 0.09 | | | | |
| v/c Ratio | 0.35 | 0.55 | | 0.67 | 0.69 | 0.02 | 0.80 | 0.30 | 0.09 | 0.61 | 0.99 | |
| Uniform Delay, d1 | 46.0 | 46.9 | | 46.0 | 46.1 | 42.3 | 49.2 | 18.3 | 16.4 | 42.9 | 25.1 | |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 0.92 | 0.77 | 0.51 | 0.83 | 0.63 | |
| Incremental Delay, d2 | 0.6 | 3.4 | | 8.3 | 8.7 | 0.0 | 23.8 | 0.5 | 0.2 | 2.2 | 11.6 | |
| Delay (s) | 46.6 | 50.3 | | 54.3 | 54.8 | 42.3 | 69.0 | 14.6 | 8.6 | 37.6 | 27.4 | |
| Level of Service | D | D | | D | D | D | E | B | A | D | C | |
| Approach Delay (s) | 48.4 | | | 53.1 | | | 22.4 | | | 28.0 | | |
| Approach LOS | D | | | D | | | C | | | C | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 30.1 | | HCM Level of Service | | | | C | | | | | |
| HCM Volume to Capacity ratio | 0.91 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | | | 16.0 | | | | | |
| Intersection Capacity Utilization | 79.9% | | ICU Level of Service | | | | D | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|------|-------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.97 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 1770 | 3423 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4837 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 1770 | 3423 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4837 | |
| Volume (vph) | 243 | 234 | 65 | 0 | 0 | 0 | 242 | 825 | 429 | 0 | 603 | 605 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 264 | 254 | 71 | 0 | 0 | 0 | 263 | 897 | 466 | 0 | 655 | 658 |
| RTOR Reduction (vph) | 0 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 79 | 0 | 0 | 370 |
| Lane Group Flow (vph) | 264 | 300 | 0 | 0 | 0 | 0 | 263 | 897 | 387 | 0 | 655 | 288 |
| Turn Type | Prot | | | Prot | | Perm | Prot | | Perm | Prot | | |
| Protected Phases | 7 | 4 | | | | | 5 | 2 | | | 6 | |
| Permitted Phases | | | | | | | | 2 | | | | 6 |
| Actuated Green, G (s) | 20.9 | 20.9 | | | | | 29.0 | 81.1 | 81.1 | | 48.1 | 48.1 |
| Effective Green, g (s) | 20.9 | 20.9 | | | | | 29.0 | 81.1 | 81.1 | | 48.1 | 48.1 |
| Actuated g/C Ratio | 0.19 | 0.19 | | | | | 0.26 | 0.74 | 0.74 | | 0.44 | 0.44 |
| Clearance Time (s) | 4.0 | 4.0 | | | | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | | | | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 336 | 650 | | | | | 467 | 2609 | 1167 | | 1548 | 692 |
| v/s Ratio Prot | c0.15 | 0.09 | | | | | c0.15 | 0.25 | | | 0.19 | |
| v/s Ratio Perm | | | | | | | | 0.29 | | | | 0.42 |
| v/c Ratio | 0.79 | 0.46 | | | | | 0.56 | 0.34 | 0.33 | | 0.42 | 0.42 |
| Uniform Delay, d1 | 42.4 | 39.6 | | | | | 35.0 | 5.1 | 5.0 | | 21.4 | 21.3 |
| Progression Factor | 1.00 | 1.00 | | | | | 0.98 | 0.72 | 0.43 | | 1.00 | 1.00 |
| Incremental Delay, d2 | 11.4 | 0.5 | | | | | 1.4 | 0.3 | 0.7 | | 0.9 | 1.8 |
| Delay (s) | 53.9 | 40.1 | | | | | 35.8 | 4.0 | 2.9 | | 22.2 | 23.1 |
| Level of Service | D | D | | | | | D | A | A | | C | C |
| Approach Delay (s) | 46.3 | | | 0.0 | | | 8.8 | | | 22.7 | | |
| Approach LOS | D | | | A | | | A | | | C | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 20.2 | | HCM Level of Service | | | | C | | | | | |
| HCM Volume to Capacity ratio | 0.80 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 57.5% | | ICU Level of Service | | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Rd

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1833 | 1583 | 1770 | 1583 | 1770 | 1583 | 3530 | 3530 | 1770 | 3539 | 1770 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.72 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.18 | 1.00 | 0.18 | 1.00 |
| Satd. Flow (perm) | 1833 | 1583 | 1342 | 1583 | 1770 | 1583 | 3530 | 3530 | 339 | 3539 | 339 | 3539 |
| Volume (vph) | 17 | 35 | 143 | 13 | 0 | 147 | 0 | 1220 | 22 | 87 | 549 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 18 | 38 | 155 | 14 | 0 | 160 | 0 | 1326 | 24 | 95 | 597 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 139 | 0 | 0 | 58 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 56 | 16 | 14 | 0 | 102 | 0 | 1349 | 0 | 95 | 597 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 2 | | | 6 | | | 6 | | |
| Permitted Phases | 4 | | | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 11.6 | 11.6 | 11.6 | 11.6 | 11.6 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 |
| Effective Green, g (s) | 11.6 | 11.6 | 11.6 | 11.6 | 11.6 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 | 90.4 |
| Actuated g/C Ratio | 0.11 | 0.11 | 0.11 | 0.11 | 0.11 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 193 | 167 | 142 | 167 | 2901 | 279 | 2908 | 2908 | 279 | 2908 | 279 | 2908 |
| v/s Ratio Prot | 0.03 | | | 0.10 | | | 0.01 | | | 0.10 | | |
| v/c Ratio | 0.29 | 0.10 | 0.10 | 0.61 | 0.47 | 0.34 | 0.21 | 0.28 | 0.34 | 0.21 | 0.28 | 0.21 |
| Uniform Delay, d1 | 45.4 | 44.5 | 44.5 | 47.0 | 2.8 | 2.4 | 2.1 | 2.8 | 2.4 | 2.1 | 2.8 | 2.1 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 2.06 | 0.35 | 1.00 | 2.06 | 0.35 | 1.00 | 0.35 |
| Incremental Delay, d2 | 0.8 | 0.3 | 0.3 | 6.2 | 0.5 | 3.1 | 0.1 | 0.8 | 0.3 | 0.3 | 6.2 | 0.1 |
| Delay (s) | 46.2 | 44.7 | 44.8 | 53.2 | 3.4 | 8.0 | 0.9 | 46.2 | 44.7 | 44.8 | 53.2 | 0.9 |
| Level of Service | D | D | D | D | A | A | A | D | D | D | D | A |
| Approach Delay (s) | 45.1 | | | 52.5 | | | 3.4 | | | 1.9 | | |
| Approach LOS | D | | | D | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 10.1 | | | HCM Level of Service | | | B | | | | | |
| HCM Volume to Capacity ratio | 0.52 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 110.0 | | | Sum of lost time (s) | | | 8.0 | | | | | |
| Intersection Capacity Utilization | 58.7% | | | ICU Level of Service | | | B | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Ave

Cumulative + Project AM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 28 | 224 | 52 | 186 | 718 | 39 | 92 | 14 | 82 | 30 | 9 | 56 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 30 | 243 | 57 | 202 | 780 | 42 | 100 | 15 | 89 | 33 | 10 | 61 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 823 | | | 300 | | | 1583 | | | 1560 | | |
| vC1, stage 1 conf vol | | | | | | | 272 | | | 1518 | | |
| vC2, stage 2 conf vol | | | | | | | 1567 | | | 802 | | |
| vCu, unblocked vol | 823 | | | 300 | | | 1583 | | | 1560 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | |
| tC, 2 stage (s) | | | | | | | 6.2 | | | 7.1 | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | |
| p0 queue free % | 96 | | | 84 | | | 0 | | | 83 | | |
| cM capacity (veh/h) | 807 | | | 1261 | | | 58 | | | 91 | | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 30 | 300 | 202 | 823 | 115 | 89 | 103 | | | | | |
| Volume Left | 30 | 0 | 202 | 0 | 100 | 0 | 33 | | | | | |
| Volume Right | 0 | 57 | 0 | 42 | 0 | 89 | 61 | | | | | |
| cSH | 807 | 1700 | 1261 | 1700 | 61 | 767 | 133 | | | | | |
| Volume to Capacity | 0.04 | 0.18 | 0.16 | 0.48 | 1.89 | 0.12 | 0.77 | | | | | |
| Queue Length (ft) | 3 | 0 | 14 | 0 | 270 | 10 | 116 | | | | | |
| Control Delay (s) | 9.6 | 0.0 | 8.4 | 0.0 | 567.1 | 10.3 | 90.8 | | | | | |
| Lane LOS | A | A | A | F | B | F | F | | | | | |
| Approach Delay (s) | 0.9 | | | 1.7 | | | 324.3 | | | 90.8 | | |
| Approach LOS | D | | | D | | | F | | | F | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 46.7 | | | | | | | | | | | |
| Intersection Capacity Utilization | 66.0% | | | ICU Level of Service | | | C | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
1: Rancho View Drive & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 0.95 | |
| Frt | 0.90 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Flt Protected | 0.99 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1651 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Flt Permitted | 0.96 | | 0.95 | | 1.00 | | 1.00 | | 1.00 | | 1.00 | |
| Satd. Flow (perm) | 1602 | | 1770 | | 3539 | | 3528 | | 3528 | | 3528 | |
| Volume (vph) | 6 | 0 | 21 | 0 | 0 | 0 | 19 | 2265 | 0 | 0 | 875 | 18 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 7 | 0 | 23 | 0 | 0 | 0 | 21 | 2462 | 0 | 0 | 951 | 20 |
| RTOR Reduction (vph) | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 |
| Lane Group Flow (vph) | 0 | 11 | 0 | 0 | 0 | 0 | 21 | 2462 | 0 | 0 | 970 | 0 |
| Turn Type | Prot | | Prot | | Prot | | Prot | | Prot | | Prot | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 6 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Effective Green, g (s) | 18.0 | | 2.8 | | 84.0 | | 77.2 | | 77.2 | | 77.2 | |
| Actuated g/C Ratio | 0.16 | | 0.03 | | 0.76 | | 0.70 | | 0.70 | | 0.70 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 262 | | 45 | | 2703 | | 2476 | | 2476 | | 2476 | |
| v/s Ratio Prot | 0.01 | | c0.70 | | 0.28 | | 0.28 | | 0.28 | | 0.28 | |
| v/s Ratio Perm | c0.02 | | 0.47 | | 0.91 | | 0.39 | | 0.39 | | 0.39 | |
| v/c Ratio | 0.04 | | 0.47 | | 0.91 | | 0.39 | | 0.39 | | 0.39 | |
| Uniform Delay, d1 | 38.7 | | 52.9 | | 10.1 | | 6.7 | | 6.7 | | 6.7 | |
| Progression Factor | 1.00 | | 0.89 | | 1.05 | | 1.00 | | 1.00 | | 1.00 | |
| Incremental Delay, d2 | 0.1 | | 3.5 | | 2.9 | | 0.5 | | 0.5 | | 0.5 | |
| Delay (s) | 38.8 | | 50.3 | | 13.5 | | 7.2 | | 7.2 | | 7.2 | |
| Level of Service | D | | D | | B | | A | | A | | A | |
| Approach Delay (s) | 38.8 | | 0.0 | | 13.9 | | 7.2 | | 7.2 | | 7.2 | |
| Approach LOS | D | | A | | B | | A | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 12.2 | | HCM Level of Service | | B | | B | | B | | B | |
| HCM Volume to Capacity ratio | 0.77 | | Sum of lost time (s) | | 8.0 | | 8.0 | | 8.0 | | 8.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | C | | C | | C | | C | |
| Intersection Capacity Utilization | 72.6% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
2: Green Valley Drive & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|-----------------------|------|------|------|------|------|-------|------|------|------|
| Lane Configurations | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | | ↔ | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Lane Util. Factor | 1.00 | | 1.00 | | 1.00 | | 0.95 | | 0.95 | | 0.95 | |
| Frt | 0.97 | | 1.00 | | 0.85 | | 1.00 | | 0.85 | | 1.00 | |
| Flt Protected | 0.96 | | 0.95 | | 1.00 | | 0.95 | | 1.00 | | 1.00 | |
| Satd. Flow (prot) | 1735 | | 1770 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Flt Permitted | 0.87 | | 0.75 | | 1.00 | | 0.95 | | 1.00 | | 0.95 | |
| Satd. Flow (perm) | 1571 | | 1397 | | 1583 | | 1770 | | 3539 | | 1583 | |
| Volume (vph) | 8 | 0 | 3 | 13 | 0 | 17 | 3 | 2213 | 23 | 12 | 853 | 4 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 9 | 0 | 3 | 14 | 0 | 18 | 3 | 2405 | 25 | 13 | 927 | 4 |
| RTOR Reduction (vph) | 0 | 3 | 0 | 0 | 0 | 15 | 0 | 0 | 5 | 0 | 0 | 1 |
| Lane Group Flow (vph) | 0 | 9 | 0 | 0 | 14 | 3 | 3 | 2405 | 20 | 13 | 927 | 3 |
| Turn Type | Prot | | Prot | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 7 | 4 | 3 | | 8 | 5 | | 2 | 1 | | 6 | |
| Permitted Phases | 7 | | 4 | | 3 | | 8 | | 5 | | 2 | |
| Actuated Green, G (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 78.8 | | 80.0 | |
| Effective Green, g (s) | 16.8 | | 16.8 | | 16.8 | | 1.2 | | 78.8 | | 80.0 | |
| Actuated g/C Ratio | 0.15 | | 0.15 | | 0.15 | | 0.01 | | 0.72 | | 0.73 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 240 | | 213 | | 242 | | 19 | | 2535 | | 1151 | |
| v/s Ratio Prot | 0.01 | | 0.01 | | 0.01 | | 0.00 | | c0.68 | | 0.26 | |
| v/s Ratio Perm | 0.01 | | 0.07 | | 0.01 | | 0.16 | | 0.95 | | 0.02 | |
| v/c Ratio | 0.04 | | 0.07 | | 0.01 | | 0.16 | | 0.95 | | 0.02 | |
| Uniform Delay, d1 | 39.7 | | 39.9 | | 39.6 | | 53.9 | | 13.8 | | 4.1 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 1.03 | | 0.83 | | 0.69 | |
| Incremental Delay, d2 | 0.1 | | 0.1 | | 0.0 | | 2.3 | | 6.1 | | 0.4 | |
| Delay (s) | 39.8 | | 40.0 | | 39.6 | | 57.6 | | 17.6 | | 3.9 | |
| Level of Service | D | | D | | D | | E | | B | | A | |
| Approach Delay (s) | 39.8 | | 39.8 | | 39.8 | | 17.5 | | 4.7 | | 4.7 | |
| Approach LOS | D | | D | | D | | B | | A | | A | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 14.2 | | HCM Level of Service | | B | | B | | B | | B | |
| HCM Volume to Capacity ratio | 0.78 | | Sum of lost time (s) | | 12.0 | | 12.0 | | 12.0 | | 12.0 | |
| Actuated Cycle Length (s) | 110.0 | | ICU Level of Service | | D | | D | | D | | D | |
| Intersection Capacity Utilization | 77.8% | | Analysis Period (min) | | 15 | | 15 | | 15 | | 15 | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
3: Reliez Valley Road & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
|-----------------------------------|-------|------|----------------------|-------|------|------|
| Lane Configurations | | | | | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Flt Permitted | 0.95 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1770 | 1583 | 1770 | 3539 | 3539 | 1583 |
| Volume (vph) | 22 | 111 | 197 | 2224 | 868 | 28 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 24 | 121 | 214 | 2417 | 943 | 30 |
| RTOR Reduction (vph) | 0 | 113 | 0 | 0 | 0 | 11 |
| Lane Group Flow (vph) | 24 | 8 | 214 | 2417 | 943 | 19 |
| Turn Type | Perm | | Prot | | Perm | |
| Protected Phases | 4 | | 5 | | 2 | |
| Permitted Phases | 4 | | 6 | | 6 | |
| Actuated Green, G (s) | 7.4 | 7.4 | 22.0 | 94.6 | 68.6 | 68.6 |
| Effective Green, g (s) | 7.4 | 7.4 | 22.0 | 94.6 | 68.6 | 68.6 |
| Actuated g/C Ratio | 0.07 | 0.07 | 0.20 | 0.86 | 0.62 | 0.62 |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Lane Grp Cap (vph) | 119 | 106 | 354 | 3044 | 2207 | 987 |
| v/s Ratio Prot | 0.01 | | 0.12 | c0.68 | 0.27 | |
| v/s Ratio Perm | | 0.08 | | | | 0.02 |
| v/c Ratio | 0.20 | 0.08 | 0.60 | 0.79 | 0.43 | 0.02 |
| Uniform Delay, d1 | 48.5 | 48.1 | 40.0 | 3.4 | 10.6 | 7.9 |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.34 | 2.96 |
| Incremental Delay, d2 | 0.8 | 0.3 | 2.9 | 2.2 | 0.6 | 0.0 |
| Delay (s) | 49.3 | 48.4 | 42.9 | 5.6 | 14.8 | 23.4 |
| Level of Service | D | D | D | A | B | C |
| Approach Delay (s) | 48.6 | | 8.7 | | 15.1 | |
| Approach LOS | D | | A | | B | |
| Intersection Summary | | | | | | |
| HCM Average Control Delay | 11.9 | | HCM Level of Service | | B | |
| HCM Volume to Capacity ratio | 0.82 | | | | | |
| Actuated Cycle Length (s) | 110.0 | | Sum of lost time (s) | | 8.0 | |
| Intersection Capacity Utilization | 71.5% | | ICU Level of Service | | C | |
| Analysis Period (min) | 15 | | | | | |
| c Critical Lane Group | | | | | | |

HCM Signalized Intersection Capacity Analysis
4: Spring Hill Road & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|------|------|------|------|-------|------|------|------|
| Lane Configurations | | | | | | | | | | | | |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Frt | 1.00 | 0.85 | 0.97 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 |
| Flt Protected | 0.96 | 1.00 | 0.96 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (prot) | 1781 | 1583 | 1741 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.79 | 1.00 | 0.75 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 |
| Satd. Flow (perm) | 1469 | 1583 | 1366 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Volume (vph) | 21 | 2 | 94 | 24 | 0 | 6 | 133 | 2407 | 36 | 15 | 1020 | 26 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 23 | 2 | 102 | 26 | 0 | 7 | 145 | 2616 | 39 | 16 | 1109 | 28 |
| RTOR Reduction (vph) | 0 | 0 | 96 | 0 | 7 | 0 | 0 | 0 | 5 | 0 | 0 | 7 |
| Lane Group Flow (vph) | 0 | 25 | 6 | 0 | 26 | 0 | 145 | 2616 | 34 | 16 | 1109 | 21 |
| Turn Type | Perm | | Perm | | Perm | | Prot | | Perm | | Perm | |
| Protected Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Permitted Phases | 4 | | 4 | | 8 | | 5 | | 2 | | 1 | |
| Actuated Green, G (s) | 8.2 | | 8.2 | | 8.2 | | 25.4 | | 126.5 | | 3.3 | |
| Effective Green, g (s) | 8.2 | | 8.2 | | 8.2 | | 25.4 | | 126.5 | | 3.3 | |
| Actuated g/C Ratio | 0.05 | | 0.05 | | 0.05 | | 0.17 | | 0.84 | | 0.02 | |
| Clearance Time (s) | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | | 4.0 | |
| Vehicle Extension (s) | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | | 3.0 | |
| Lane Grp Cap (vph) | 80 | | 87 | | 75 | | 300 | | 2985 | | 1102 | |
| v/s Ratio Prot | 0.02 | | 0.06 | | 0.02 | | 0.08 | | c0.74 | | 0.01 | |
| v/s Ratio Perm | 0.31 | | 0.06 | | 0.35 | | 0.48 | | 0.88 | | 0.03 | |
| v/c Ratio | 0.31 | | 0.06 | | 0.35 | | 0.48 | | 0.88 | | 0.03 | |
| Uniform Delay, d1 | 68.2 | | 67.3 | | 68.3 | | 56.4 | | 7.1 | | 1.9 | |
| Progression Factor | 1.00 | | 1.00 | | 1.00 | | 0.77 | | 0.14 | | 0.00 | |
| Incremental Delay, d2 | 2.2 | | 0.3 | | 2.8 | | 0.1 | | 0.4 | | 0.0 | |
| Delay (s) | 70.4 | | 67.6 | | 71.2 | | 43.4 | | 1.4 | | 0.0 | |
| Level of Service | E | | E | | E | | D | | A | | A | |
| Approach Delay (s) | 68.1 | | 71.2 | | 71.2 | | 3.5 | | 3.5 | | 11.6 | |
| Approach LOS | E | | E | | E | | A | | A | | B | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 8.3 | | | HCM Level of Service | | | A | | | | | |
| HCM Volume to Capacity ratio | 0.89 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 12.0 | | | | | |
| Intersection Capacity Utilization | 88.2% | | | ICU Level of Service | | | E | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
5: Deer Hill Road & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|-------|------|------|------|-------|------|-------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 0.97 | 1.00 | | 0.95 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.91 | |
| Frt | 1.00 | 0.90 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.98 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 3433 | 1679 | | 1681 | 1748 | 1583 | 1770 | 3539 | 1583 | 1770 | 4999 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 0.99 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 3433 | 1679 | | 1681 | 1748 | 1583 | 1770 | 3539 | 1583 | 1770 | 4999 | |
| Volume (vph) | 550 | 75 | 145 | 114 | 72 | 121 | 395 | 2018 | 78 | 93 | 869 | 110 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 598 | 82 | 158 | 124 | 78 | 132 | 429 | 2193 | 85 | 101 | 945 | 120 |
| RTOR Reduction (vph) | 0 | 46 | 0 | 0 | 0 | 75 | 0 | 0 | 19 | 0 | 10 | 0 |
| Lane Group Flow (vph) | 598 | 194 | 0 | 98 | 104 | 57 | 429 | 2193 | 66 | 101 | 1055 | 0 |
| Turn Type | Split | | | Split | | Perm | Prot | | Perm | Prot | | |
| Protected Phases | 4 | 4 | | 8 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | | | | | 8 | | | 2 | | | 6 | |
| Actuated Green, G (s) | 25.0 | 25.0 | | 13.3 | 13.3 | 13.3 | 47.0 | 87.7 | 87.7 | 8.0 | 48.7 | |
| Effective Green, g (s) | 25.0 | 25.0 | | 13.3 | 13.3 | 13.3 | 47.0 | 87.7 | 87.7 | 8.0 | 48.7 | |
| Actuated g/C Ratio | 0.17 | 0.17 | | 0.09 | 0.09 | 0.09 | 0.31 | 0.58 | 0.58 | 0.05 | 0.32 | |
| Clearance Time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 572 | 280 | | 149 | 155 | 140 | 555 | 2069 | 926 | 94 | 1623 | |
| v/s Ratio Prot | c0.17 | 0.14 | | 0.06 | 0.06 | | 0.24 | c0.62 | | c0.06 | 0.21 | |
| v/s Ratio Perm | | | | | 0.08 | | | 0.05 | | | | |
| v/c Ratio | 1.05 | 0.69 | | 0.66 | 0.67 | 0.41 | 0.77 | 1.06 | 0.07 | 1.07 | 0.65 | |
| Uniform Delay, d1 | 62.5 | 58.9 | | 66.1 | 66.2 | 64.6 | 46.7 | 31.1 | 13.5 | 71.0 | 43.4 | |
| Progression Factor | 1.00 | 1.00 | | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | |
| Incremental Delay, d2 | 50.0 | 7.2 | | 10.0 | 10.9 | 1.9 | 6.6 | 37.9 | 0.1 | 114.4 | 2.0 | |
| Delay (s) | 112.5 | 66.1 | | 76.2 | 77.1 | 66.6 | 53.3 | 69.1 | 13.7 | 185.4 | 45.4 | |
| Level of Service | F | E | | E | E | E | D | E | B | F | D | |
| Approach Delay (s) | 99.2 | | | | 72.7 | | 64.8 | | | | 57.5 | |
| Approach LOS | F | | | | E | | E | | | | E | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 69.4 | | HCM Level of Service | | E | | | | | | | |
| HCM Volume to Capacity ratio | 1.05 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 16.0 | | | | | | | |
| Intersection Capacity Utilization | 95.0% | | ICU Level of Service | | F | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
6: Mt. Diablo Blvd & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|----------------------|------|------|-------|------|------|------|------|------|------|
| Lane Configurations | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ | ↔ | | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 0.95 | | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.93 | | 1.00 | 1.00 | 0.85 | 1.00 | 1.00 | 0.85 | 1.00 | 0.85 | |
| Flt Protected | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (prot) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4999 | |
| Flt Permitted | 0.95 | 1.00 | | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | |
| Satd. Flow (perm) | 1770 | 3304 | | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 4999 | |
| Volume (vph) | 446 | 261 | 208 | 0 | 0 | 0 | 224 | 1096 | 445 | 0 | 733 | 390 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 485 | 284 | 226 | 0 | 0 | 0 | 243 | 1191 | 484 | 0 | 797 | 424 |
| RTOR Reduction (vph) | 0 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 148 | 0 | 0 | 234 |
| Lane Group Flow (vph) | 485 | 399 | 0 | 0 | 0 | 0 | 243 | 1191 | 336 | 0 | 797 | 190 |
| Turn Type | Prot | | | Prot | | Perm | Prot | | Perm | Prot | | Perm |
| Protected Phases | 7 | 4 | | | | 5 | 2 | | | 6 | | |
| Permitted Phases | | | | | | | | | 2 | | | 6 |
| Actuated Green, G (s) | 44.6 | 44.6 | | | | 37.0 | 97.4 | 97.4 | | 56.4 | 56.4 | |
| Effective Green, g (s) | 44.6 | 44.6 | | | | 37.0 | 97.4 | 97.4 | | 56.4 | 56.4 | |
| Actuated g/C Ratio | 0.30 | 0.30 | | | | 0.25 | 0.65 | 0.65 | | 0.38 | 0.38 | |
| Clearance Time (s) | 4.0 | 4.0 | | | | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | |
| Vehicle Extension (s) | 3.0 | 3.0 | | | | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | |
| Lane Grp Cap (vph) | 526 | 982 | | | | 437 | 2298 | 1028 | | 1331 | 595 | |
| v/s Ratio Prot | c0.27 | 0.15 | | | | c0.14 | 0.34 | | | 0.23 | | |
| v/s Ratio Perm | | | | | | | | | 0.31 | | | 0.27 |
| v/c Ratio | 0.92 | 0.41 | | | | 0.56 | 0.52 | 0.33 | | 0.60 | 0.32 | |
| Uniform Delay, d1 | 51.0 | 42.1 | | | | 49.3 | 13.9 | 11.7 | | 37.7 | 33.2 | |
| Progression Factor | 1.00 | 1.00 | | | | 0.95 | 0.83 | 0.36 | | 0.34 | 0.13 | |
| Incremental Delay, d2 | 21.8 | 0.3 | | | | 1.4 | 0.7 | 0.7 | | 1.3 | 0.9 | |
| Delay (s) | 72.8 | 42.4 | | | | 48.2 | 12.2 | 4.9 | | 14.2 | 5.4 | |
| Level of Service | E | D | | | | D | B | A | | B | A | |
| Approach Delay (s) | 57.2 | | | | 0.0 | | 15.0 | | | | 11.1 | |
| Approach LOS | E | | | | A | | B | | | | B | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 24.0 | | HCM Level of Service | | C | | | | | | | |
| HCM Volume to Capacity ratio | 0.74 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | Sum of lost time (s) | | 12.0 | | | | | | | |
| Intersection Capacity Utilization | 67.4% | | ICU Level of Service | | C | | | | | | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Signalized Intersection Capacity Analysis
7: EB 24 Off Ramp & Pleasant Hill Rd

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|--------|----------------------|--------|------|-------|------|------|-------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Total Lost time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Frt | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 0.85 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Flt Protected | 0.98 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.95 | 1.00 | 0.95 | 1.00 |
| Satd. Flow (prot) | 1830 | 1583 | 1770 | 1583 | 1583 | 1583 | 3529 | 3529 | 1770 | 3539 | 1770 | 3539 |
| Flt Permitted | 0.98 | 1.00 | 0.43 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.12 | 1.00 | 0.12 | 1.00 |
| Satd. Flow (perm) | 1830 | 1583 | 804 | 1583 | 1583 | 1583 | 3529 | 3529 | 227 | 3539 | 227 | 3539 |
| Volume (vph) | 38 | 69 | 189 | 18 | 0 | 34 | 0 | 1531 | 29 | 131 | 690 | 0 |
| Peak-hour factor, PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 41 | 75 | 205 | 20 | 0 | 37 | 0 | 1664 | 32 | 142 | 750 | 0 |
| RTOR Reduction (vph) | 0 | 0 | 185 | 0 | 0 | 33 | 0 | 1 | 0 | 0 | 0 | 0 |
| Lane Group Flow (vph) | 0 | 116 | 20 | 20 | 0 | 4 | 0 | 1695 | 0 | 142 | 750 | 0 |
| Turn Type | Perm | Perm | custom | custom | custom | Perm | Perm | Perm | Perm | Perm | Perm | Perm |
| Protected Phases | 4 | | | 8 | | | 2 | | | 6 | | |
| Permitted Phases | 4 | 4 | 8 | 8 | | | 6 | | | 6 | | |
| Actuated Green, G (s) | 14.5 | 14.5 | 14.5 | 14.5 | | | 127.5 | | | 127.5 | | |
| Effective Green, g (s) | 14.5 | 14.5 | 14.5 | 14.5 | | | 127.5 | | | 127.5 | | |
| Actuated g/C Ratio | 0.10 | 0.10 | 0.10 | 0.10 | | | 0.85 | | | 0.85 | | |
| Clearance Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | | | 4.0 | | | 4.0 | | |
| Vehicle Extension (s) | 3.0 | 3.0 | 3.0 | 3.0 | | | 3.0 | | | 3.0 | | |
| Lane Grp Cap (vph) | 177 | 153 | 78 | 153 | | | 3000 | | | 193 | | |
| v/s Ratio Prot | | | | 0.48 | | | 0.48 | | | 0.21 | | |
| v/s Ratio Perm | 0.06 | 0.13 | 0.02 | 0.02 | | | c0.63 | | | c0.63 | | |
| v/c Ratio | 0.66 | 0.13 | 0.26 | 0.02 | | | 0.57 | | | 0.74 | | |
| Uniform Delay, d1 | 65.3 | 62.0 | 62.8 | 61.3 | | | 3.2 | | | 4.5 | | |
| Progression Factor | 1.00 | 1.00 | 1.00 | 1.00 | | | 1.00 | | | 2.02 | | |
| Incremental Delay, d2 | 8.4 | 0.4 | 1.7 | 0.1 | | | 0.8 | | | 18.9 | | |
| Delay (s) | 73.8 | 62.4 | 64.5 | 61.4 | | | 4.0 | | | 28.0 | | |
| Level of Service | E | E | E | E | | | A | | | C | | |
| Approach Delay (s) | 66.5 | | | 62.5 | | | 4.0 | | | 5.1 | | |
| Approach LOS | E | | | E | | | A | | | A | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM Average Control Delay | 12.2 | | | HCM Level of Service | | | B | | | B | | |
| HCM Volume to Capacity ratio | 0.80 | | | | | | | | | | | |
| Actuated Cycle Length (s) | 150.0 | | | Sum of lost time (s) | | | 8.0 | | | 8.0 | | |
| Intersection Capacity Utilization | 72.9% | | | ICU Level of Service | | | C | | | C | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |
| c Critical Lane Group | | | | | | | | | | | | |

HCM Unsignalized Intersection Capacity Analysis
8: Deer Hill Road & Brown Ave

Cumulative + Project PM
6/17/2011

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|-----------------------------------|-------|------|------|----------------------|-------|------|-------|------|------|------|------|------|
| Lane Configurations | | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ | ↔ |
| Sign Control | Free | | | Free | | | Stop | | | Stop | | |
| Grade | 0% | | | 0% | | | 0% | | | 0% | | |
| Volume (veh/h) | 50 | 682 | 53 | 113 | 329 | 92 | 62 | 9 | 116 | 66 | 14 | 21 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 54 | 741 | 58 | 123 | 358 | 100 | 67 | 10 | 126 | 72 | 15 | 23 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | None | | | None | | |
| Median storage (veh) | | | | | | | | | | | | |
| Upstream signal (ft) | | | | | | | | | | | | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 458 | | | 799 | | | 1512 | | | 1582 | | |
| vC1, stage 1 conf vol | | | | | | | 770 | | | 1508 | | |
| vC2, stage 2 conf vol | | | | | | | 1561 | | | 408 | | |
| vCu, unblocked vol | 458 | | | 799 | | | 1512 | | | 1582 | | |
| tC, single (s) | 4.1 | | | 4.1 | | | 7.1 | | | 6.5 | | |
| tC, 2 stage (s) | | | | | | | 6.2 | | | 7.1 | | |
| tF (s) | 2.2 | | | 2.2 | | | 3.5 | | | 4.0 | | |
| p0 queue free % | 95 | | | 85 | | | 5 | | | 89 | | |
| cM capacity (veh/h) | 1103 | | | 824 | | | 71 | | | 88 | | |
| Direction, Lane # | EB 1 | EB 2 | WB 1 | WB 2 | NB 1 | NB 2 | SB 1 | | | | | |
| Volume Total | 54 | 799 | 123 | 458 | 77 | 126 | 110 | | | | | |
| Volume Left | 54 | 0 | 123 | 0 | 67 | 0 | 72 | | | | | |
| Volume Right | 0 | 58 | 0 | 100 | 0 | 126 | 23 | | | | | |
| cSH | 1103 | 1700 | 824 | 1700 | 73 | 401 | 71 | | | | | |
| Volume to Capacity | 0.05 | 0.47 | 0.15 | 0.27 | 1.06 | 0.31 | 1.55 | | | | | |
| Queue Length (ft) | 4 | 0 | 13 | 0 | 142 | 33 | 233 | | | | | |
| Control Delay (s) | 8.4 | 0.0 | 10.1 | 0.0 | 223.2 | 18.1 | 406.5 | | | | | |
| Lane LOS | A | B | B | F | C | F | F | | | | | |
| Approach Delay (s) | 0.5 | | 2.1 | | 95.9 | | 406.5 | | | | | |
| Approach LOS | A | | B | | F | | F | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | 37.7 | | | | | | | | | | | |
| Intersection Capacity Utilization | 67.7% | | | ICU Level of Service | | | C | | | C | | |
| Analysis Period (min) | 15 | | | | | | | | | | | |

Impact Analysis Report
Level Of Service

| Intersection | Base | | Future | | Change in |
|--------------|-----------------|---------|-----------------|---------|--------------|
| | Del/ LOS Veh | V/ C | Del/ LOS Veh | V/ C | |
| # 1 | A xxxxx | 0.486 | A xxxxx | 0.486 | + 0.000 V/C |
| # 2 | A xxxxx | 0.528 | A xxxxx | 0.528 | + 0.000 V/C |
| # 3 | B xxxxx | 0.619 | B xxxxx | 0.619 | + 0.000 V/C |
| # 4 | C xxxxx | 0.741 | C xxxxx | 0.741 | + 0.000 V/C |
| # 5 | C xxxxx | 0.725 | C xxxxx | 0.725 | + 0.000 V/C |
| # 6 | A xxxxx | 0.457 | A xxxxx | 0.457 | + 0.000 V/C |
| # 7 | A xxxxx | 0.552 | A xxxxx | 0.552 | + 0.000 V/C |

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #1
*****
Cycle (sec):          100          Critical Vol./Cap.(X):          0.486
Loss Time (sec):      0 (Y+R=4.0 sec)  Average Delay (sec/veh):      xxxxxx
Optimal Cycle:        36          Level Of Service:          A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|-----|
Control:      Protected      Protected      Permitted      Permitted
Rights:      Include      Include      Include      Include
Min. Green:      0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:      1 0 1 1 0 0 0 1 1 0 0 0 0 0 1 0 0 0
-----|-----|-----|-----|-----|
Volume Module:
Base Vol:      16 633 0 0 0 1589 10 0 0 37 0 0 0 0
Growth Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:    16 633 0 0 0 1589 10 0 0 37 0 0 0 0
User Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:    16 633 0 0 0 1589 10 0 0 37 0 0 0 0
Reduct Vol:    0 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:   16 633 0 0 0 1589 10 0 0 37 0 0 0 0
RTOR Reduct:   0 0 0 0 0 0 0 0 0 16 0 0 0 0
RTOR Vol:      16 633 0 0 0 1589 10 0 0 21 0 0 0 0
PCE Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:   16 633 0 0 0 1589 10 0 0 21 0 0 0 0
-----|-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:      1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:         1.00 2.00 0.00 0.00 1.99 0.01 0.00 0.00 1.00 0.00 1.00 0.00
Final Sat.:    1720 3440 0 0 3418 22 0 0 1720 0 1720 0
-----|-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:      0.01 0.18 0.00 0.00 0.46 0.46 0.00 0.00 0.01 0.00 0.00 0.00
Crit Volume:   16 800 21 0
Crit Moves:    ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #2
*****
Cycle (sec):      100          Critical Vol./Cap.(X):      0.528
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   39          Level Of Service:          A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Protected      Protected      Permitted      Permitted
Rights:      Include      Ignore      Include      Ignore
Min. Green:    0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:        1 0 2 0 1 1 0 2 0 1 0 0 1 0 0 0 0 1 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol:      0 710 13 8 1767 1 2 0 5 18 0 10
Growth Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:   0 710 13 8 1767 1 2 0 5 18 0 10
User Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:   0 710 13 8 1767 1 2 0 5 18 0 10
Reduct Vol:   0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:  0 710 13 8 1767 1 2 0 5 18 0 10
RTOR Reduct:  0 0 13 0 0 0 0 0 0 0 0 0
RTOR Vol:     0 710 0 8 1767 1 2 0 5 18 0 10
PCE Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:  0 710 0 8 1767 1 2 0 5 18 0 10
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:     1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:        1.00 2.00 1.00 1.00 2.00 1.00 0.29 0.00 0.71 1.00 0.00 1.00
Final Sat.:   1720 3440 1720 1720 3440 1720 491 0 1229 1720 0 1720
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:      0.00 0.21 0.00 0.00 0.51 0.00 0.00 0.00 0.00 0.01 0.00 0.01
Crit Volume:  0 884 7 18
Crit Moves:   ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #3
*****
Cycle (sec):      100          Critical Vol./Cap.(X):      0.619
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   60          Level Of Service:          B
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Protected      Protected      Protected      Protected
Rights:      Include      Include      Ignore      Include
Min. Green:    0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:        1 0 2 0 0 0 0 2 0 1 1 0 0 0 1 0 0 0 0 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol:     102 712 0 0 1878 11 24 0 242 0 0 0
Growth Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:   102 712 0 0 1878 11 24 0 242 0 0 0
User Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:   102 712 0 0 1878 11 24 0 242 0 0 0
Reduct Vol:   0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:  102 712 0 0 1878 11 24 0 242 0 0 0
RTOR Reduct:  0 0 0 0 0 0 11 0 0 0 0 0
RTOR Vol:     102 712 0 0 1878 0 24 0 242 0 0 0
PCE Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:  102 712 0 0 1878 0 24 0 242 0 0 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:     1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:        1.00 2.00 0.00 0.00 2.00 1.00 1.00 0.00 1.00 0.00 0.00 0.00
Final Sat.:   1720 3440 0 0 3440 1720 1720 0 1720 0 0 0
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:      0.06 0.21 0.00 0.00 0.55 0.00 0.01 0.00 0.14 0.00 0.00 0.00
Crit Volume:  102 939 24 0
Crit Moves:   ****
*****

```


Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #4
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.741
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   72      Level Of Service:      C
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
Control:      Protected      Protected      Permitted      Permitted
Rights:      Include      Include      Ignore      Include
Min. Green:    0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:        1 0 2 0 1 1 0 2 0 1 0 1 0 0 1 0 0 1 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol:      102 712 27 11 2176 22 28 3 148 25 3 29
Growth Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:   102 712 27 11 2176 22 28 3 148 25 3 29
User Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:   102 712 27 11 2176 22 28 3 148 25 3 29
Reduct Vol:   0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:  102 712 27 11 2176 22 28 3 148 25 3 29
RTOR Reduct:  0 0 25 0 0 22 0 0 0 0 0 0
RTOR Vol:     102 712 2 11 2176 0 28 3 148 25 3 29
PCE Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:  102 712 2 11 2176 0 28 3 148 25 3 29
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:     1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:        1.00 2.00 1.00 1.00 2.00 1.00 0.90 0.10 1.00 0.44 0.05 0.51
Final Sat.:   1720 3440 1720 1720 3440 1720 1554 166 1720 754 91 875
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:      0.06 0.21 0.00 0.01 0.63 0.00 0.02 0.02 0.09 0.03 0.03 0.03
Crit Volume:  102      1088      28      57
Crit Moves:   ****      ****      ****      ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #5
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.725
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   83      Level Of Service:      C
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
Control:      Protected      Protected      Split Phase      Split Phase
Rights:      Include      Include      Include      Include
Min. Green:    0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:        1 0 2 0 1 1 0 2 0 1 2 0 0 1 0 1 1 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol:      117 460 135 160 1643 793 115 67 44 186 80 35
Growth Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:   117 460 135 160 1643 793 115 67 44 186 80 35
User Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:   117 460 135 160 1643 793 115 67 44 186 80 35
Reduct Vol:   0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:  117 460 135 160 1643 793 115 67 44 186 80 35
RTOR Reduct:  0 0 102 0 0 63 0 0 0 0 0 0
RTOR Vol:     117 460 33 160 1643 730 115 67 44 186 80 0
PCE Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:  117 460 33 160 1643 730 115 67 44 186 80 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:     1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment:   1.00 1.00 1.00 1.00 1.00 1.00 0.91 1.00 1.00 0.91 1.00 1.00
Lanes:        1.00 2.00 1.00 1.00 2.00 1.00 2.00 0.60 0.40 1.40 0.60 1.00
Final Sat.:   1650 3300 1650 1650 3300 1650 3000 996 654 2098 992 1650
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:      0.07 0.14 0.02 0.10 0.50 0.44 0.04 0.07 0.07 0.09 0.08 0.00
Crit Volume:  117      822      111 133
Crit Moves:   ****      ****      ****      ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #6
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.457
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   42      Level Of Service:      A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Protected      Protected      Protected      Protected
Rights:      Include      Ignore      Include      Include
Min. Green:   0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:      1 0 2 0 1 0 0 2 0 1 1 0 1 1 0 0 0 0 0 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol:     242 825 429 0 603 605 243 234 65 0 0 0 0 0
Growth Adj:  1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:  242 825 429 0 603 605 243 234 65 0 0 0 0 0
User Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:  242 825 429 0 603 605 243 234 65 0 0 0 0 0
Reduct Vol:  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 242 825 429 0 603 605 243 234 65 0 0 0 0 0
RTOR Reduct: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
RTOR Vol:    242 825 429 0 603 605 243 234 65 0 0 0 0 0
PCE Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 242 825 429 0 603 605 243 234 65 0 0 0 0 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:    1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:  1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:       1.00 2.00 1.00 0.00 2.00 1.00 1.00 1.57 0.43 0.00 0.00 0.00 0.00
Final Sat.:  1720 3440 1720 0 3440 1720 1720 2692 748 0 0 0 0 0
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:     0.14 0.24 0.25 0.00 0.18 0.35 0.14 0.09 0.09 0.00 0.00 0.00 0.00
Crit Volume: 242 302 243 0
Crit Moves:  ****  ****  ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

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*****
Intersection #7
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.552
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   51      Level Of Service:      A
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Protected      Protected      Split Phase      Split Phase
Rights:      Include      Include      Include      Include
Min. Green:   0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:      0 0 1 1 0 1 0 2 0 0 0 1 0 0 1 1 0 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol:     0 1220 22 87 549 0 17 35 143 13 0 147
Growth Adj:  1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:  0 1220 22 87 549 0 17 35 143 13 0 147
User Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:  0 1220 22 87 549 0 17 35 143 13 0 147
Reduct Vol:  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1220 22 87 549 0 17 35 143 13 0 147
RTOR Reduct: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 87
RTOR Vol:    0 1220 22 87 549 0 17 35 143 13 0 147
PCE Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:     1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1220 22 87 549 0 17 35 143 13 0 147
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:    1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment:  1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:       0.00 1.96 0.04 1.00 2.00 0.00 0.33 0.67 1.00 1.00 0.00 1.00
Final Sat.:  0 3242 58 1650 3300 0 539 1111 1650 1650 0 1650
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:     0.00 0.38 0.38 0.05 0.17 0.00 0.03 0.03 0.09 0.01 0.00 0.04
Crit Volume: 621 87 143 60
Crit Moves:  ****  ****  ****  ****
*****

```

Impact Analysis Report
Level Of Service

| Intersection | Base | | Future | | Change in |
|--------------|-----------------|---------|-----------------|---------|--------------|
| | Del/ LOS Veh | V/ C | Del/ LOS Veh | V/ C | |
| # 1 | B xxxxx | 0.674 | B xxxxx | 0.674 | + 0.000 V/C |
| # 2 | B xxxxx | 0.664 | B xxxxx | 0.664 | + 0.000 V/C |
| # 3 | B xxxxx | 0.659 | B xxxxx | 0.659 | + 0.000 V/C |
| # 4 | C xxxxx | 0.738 | C xxxxx | 0.738 | + 0.000 V/C |
| # 5 | E xxxxx | 0.913 | E xxxxx | 0.913 | + 0.000 V/C |
| # 6 | B xxxxx | 0.603 | B xxxxx | 0.603 | + 0.000 V/C |
| # 7 | B xxxxx | 0.678 | B xxxxx | 0.678 | + 0.000 V/C |

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #1
*****
Cycle (sec):          100          Critical Vol./Cap.(X):          0.674
Loss Time (sec):      0 (Y+R=4.0 sec)  Average Delay (sec/veh):      xxxxxx
Optimal Cycle:        57          Level Of Service:          B
*****
Approach:      North Bound      South Bound      East Bound      West Bound
Movement:      L - T - R      L - T - R      L - T - R      L - T - R
-----|-----|-----|-----|
Control:      Protected      Protected      Permitted      Permitted
Rights:      Include      Include      Include      Include
Min. Green:    0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:        1 0 1 1 0 0 0 1 1 0 0 0 1! 0 0 0 0 1! 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol:      19 2265 0 0 875 18 6 0 21 0 0 0 0
Growth Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse:    19 2265 0 0 875 18 6 0 21 0 0 0 0
User Adj:      1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:       1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume:    19 2265 0 0 875 18 6 0 21 0 0 0 0
Reduct Vol:    0 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol:   19 2265 0 0 875 18 6 0 21 0 0 0 0
RTOR Reduct:   0 0 0 0 0 0 0 0 0 0 0 0 0 0
RTOR Vol:      19 2265 0 0 875 18 6 0 21 0 0 0 0
PCE Adj:       1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:       1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume:   19 2265 0 0 875 18 6 0 21 0 0 0 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:      1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:         1.00 2.00 0.00 0.00 1.96 0.04 0.22 0.00 0.78 0.00 1.00 0.00
Final Sat.:    1720 3440 0 0 3371 69 382 0 1338 0 1720 0
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:       0.01 0.66 0.00 0.00 0.26 0.26 0.02 0.00 0.02 0.00 0.00 0.00
Crit Volume:   1133 0 0 0 0 0 27 0
Crit Moves:    ****  ****  ****
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

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*****
Intersection #2
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.664
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   55      Level Of Service:      B
*****
Approach:  North Bound  South Bound  East Bound  West Bound
Movement:  L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:   Protected   Protected   Permitted   Permitted
Rights:    Include     Ignore     Include     Ignore
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:     1 0 2 0 1 1 0 2 0 1 0 0 1 0 0 0 0 1 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol:   3 2213 23 12 853 4 8 0 3 13 0 0 17
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 3 2213 23 12 853 4 8 0 3 13 0 0 17
User Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 3 2213 23 12 853 4 8 0 3 13 0 0 17
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 3 2213 23 12 853 4 8 0 3 13 0 0 17
RTOR Reduct: 0 0 13 0 0 0 0 0 0 0 0 0 0
RTOR Vol:   3 2213 10 12 853 4 8 0 3 13 0 0 17
PCE Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 3 2213 10 12 853 4 8 0 3 13 0 0 17
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:  1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:     1.00 2.00 1.00 1.00 2.00 1.00 0.73 0.00 0.27 1.00 0.00 1.00
Final Sat.: 1720 3440 1720 1720 3440 1720 1251 0 469 1720 0 1720
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:   0.00 0.64 0.01 0.01 0.25 0.00 0.01 0.00 0.01 0.01 0.00 0.01
Crit Volume: 1107 12 11 13
Crit Moves: **** **
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

```

*****
Intersection #3
*****
Cycle (sec):      100      Critical Vol./Cap.(X):      0.659
Loss Time (sec):  0 (Y+R=4.0 sec) Average Delay (sec/veh):  xxxxxx
Optimal Cycle:   67      Level Of Service:      B
*****
Approach:  North Bound  South Bound  East Bound  West Bound
Movement:  L - T - R    L - T - R    L - T - R    L - T - R
-----|-----|-----|-----|
Control:   Protected   Protected   Protected   Protected
Rights:    Include     Include     Ignore     Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes:     1 0 2 0 0 0 0 2 0 1 1 0 0 0 1 0 0 0 0 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol:   197 2224 0 0 868 28 22 0 111 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 197 2224 0 0 868 28 22 0 111 0 0 0 0
User Adj:   1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 197 2224 0 0 868 28 22 0 111 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 197 2224 0 0 868 28 22 0 111 0 0 0 0
RTOR Reduct: 0 0 0 0 0 0 22 0 0 0 0 0 0
RTOR Vol:   197 2224 0 0 868 6 22 0 111 0 0 0 0
PCE Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj:    1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 197 2224 0 0 868 6 22 0 111 0 0 0 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane:  1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes:     1.00 2.00 0.00 0.00 2.00 1.00 1.00 0.00 1.00 0.00 0.00 0.00
Final Sat.: 1720 3440 0 0 3440 1720 1720 0 1720 0 0 0 0
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat:   0.11 0.65 0.00 0.00 0.25 0.00 0.01 0.00 0.06 0.00 0.00 0.00
Crit Volume: 1112 0 22
Crit Moves: **** **
*****

```

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #4

Cycle (sec): 100 Critical Vol./Cap.(X): 0.738
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 71 Level Of Service: C

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|
Control: Protected Protected Permitted Permitted
Rights: Include Include Ignore Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 0 1 0 0 1 0 0 1 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol: 133 2407 36 15 1020 26 21 2 94 24 0 6
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 133 2407 36 15 1020 26 21 2 94 24 0 6
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 133 2407 36 15 1020 26 21 2 94 24 0 6
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 133 2407 36 15 1020 26 21 2 94 24 0 6
RTOR Reduct: 0 0 24 0 0 21 0 0 0 0 0 0
RTOR Vol: 133 2407 12 15 1020 5 21 2 94 24 0 6
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 133 2407 12 15 1020 5 21 2 94 24 0 6
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane: 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 0.91 0.09 1.00 0.80 0.00 0.20
Final Sat.: 1720 3440 1720 1720 3440 1720 1570 150 1720 1376 0 344
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat: 0.08 0.70 0.01 0.01 0.30 0.00 0.01 0.01 0.05 0.02 0.00 0.02
Crit Volume: 1204 15 21 30
Crit Moves: **** **

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #5

Cycle (sec): 100 Critical Vol./Cap.(X): 0.913
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|
Control: Protected Protected Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 2 0 0 1 0 1 1 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol: 395 2018 78 93 869 110 550 75 145 114 72 121
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 395 2018 78 93 869 110 550 75 145 114 72 121
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 395 2018 78 93 869 110 550 75 145 114 72 121
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 395 2018 78 93 869 110 550 75 145 114 72 121
RTOR Reduct: 0 0 63 0 0 110 0 0 0 0 0 0
RTOR Vol: 395 2018 15 93 869 0 550 75 145 114 72 28
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 395 2018 15 93 869 0 550 75 145 114 72 28
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 0.91 1.00 1.00 0.91 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 2.00 0.34 0.66 1.23 0.77 1.00
Final Sat.: 1650 3300 1650 1650 3300 1650 3000 563 1087 1839 1277 1650
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat: 0.24 0.61 0.01 0.06 0.26 0.00 0.18 0.13 0.13 0.06 0.06 0.02
Crit Volume: 1009 93 275 93
Crit Moves: **** **

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #6

Cycle (sec): 100 Critical Vol./Cap.(X): 0.603
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 57 Level Of Service: B

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|
Control: Protected Protected Protected Protected
Rights: Include Ignore Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 0 0 2 0 1 1 0 1 0 0 0 0 0 0
-----|-----|-----|-----|
Volume Module:
Base Vol: 224 1096 445 0 733 390 446 261 208 0 0 0 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 224 1096 445 0 733 390 446 261 208 0 0 0 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 224 1096 445 0 733 390 446 261 208 0 0 0 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 224 1096 445 0 733 390 446 261 208 0 0 0 0
RTOR Reduct: 0 0 0 0 0 0 0 0 0 0 0 0 0
RTOR Vol: 224 1096 445 0 733 390 446 261 208 0 0 0 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 224 1096 445 0 733 390 446 261 208 0 0 0 0
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane: 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720 1720
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 0.00 2.00 1.00 1.00 1.11 0.89 0.00 0.00 0.00
Final Sat.: 1720 3440 1720 0 3440 1720 1720 1914 1526 0 0 0 0
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat: 0.13 0.32 0.26 0.00 0.21 0.23 0.26 0.14 0.14 0.00 0.00 0.00
Crit Volume: 224 367 446 0
Crit Moves: ****

Level Of Service Computation Report
CCTALOS Method (Base Volume Alternative)

Intersection #7

Cycle (sec): 100 Critical Vol./Cap.(X): 0.678
Loss Time (sec): 0 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 71 Level Of Service: B

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
-----|-----|-----|-----|
Control: Protected Protected Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1 1 0 1 0 2 0 0 0 1 0 0 1 1 0 0 0 1
-----|-----|-----|-----|
Volume Module:
Base Vol: 0 1531 29 131 690 0 38 69 189 18 0 34
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 1531 29 131 690 0 38 69 189 18 0 34
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1531 29 131 690 0 38 69 189 18 0 34
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1531 29 131 690 0 38 69 189 18 0 34
RTOR Reduct: 0 0 0 0 0 0 0 0 0 0 0 0
RTOR Vol: 0 1531 29 131 690 0 38 69 189 18 0 34
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1531 29 131 690 0 38 69 189 18 0 34
-----|-----|-----|-----|
Saturation Flow Module:
Sat/Lane: 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650 1650
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 1.96 0.04 1.00 2.00 0.00 0.36 0.64 1.00 1.00 0.00 1.00
Final Sat.: 0 3239 61 1650 3300 0 586 1064 1650 1650 0 1650
-----|-----|-----|-----|
Capacity Analysis Module:
Vol/Sat: 0.00 0.47 0.47 0.08 0.21 0.00 0.06 0.06 0.11 0.01 0.00 0.00
Crit Volume: 780 131 189 18
Crit Moves: ****
